

Surface Water Management Plan

Llangatwg School Development

1 September 2025

Morgan Sindall Construction &
Infrastructure Limited

Llangatwg Comprehensive School
Main Road
Aberdulais
Neath
SA10 8DB



Document Control

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1 Introduction

1.1 Authorisation and Context

Ashfield Solutions Limited ("Ashfield") have been commissioned by Morgan Sindall Construction & Infrastructure Limited ("Morgan Sindall" or "the client"), to provide a Surface Water Management Plan ("SWMP") for the construction-phase of the proposed re-development of Llangatwg Community School within the Aberdulais area of Neath, South Wales, SA10 8DB ("the site"). A site location drawing is presented within Appendix A.

1.2 Aim and Objectives

The aim of this SWMP is to outline how surface water is managed throughout the construction phase of the site development, to minimise any potential impact on local receptors/environment. This has been achieved through meeting the following objectives:

- Outline roles and responsibilities regarding the SWMP during the project;
- Outline consideration for pollution prevention and water treatment on site; and,
- Outline consideration for flood and weather alerts and registration to these services.

Important: Ashfield understands that the upcoming Sustainable Drainage Systems Approving Body ("SAB") application will involve the conditioning of a SWMP. Accordingly, this report will fulfil two roles; 1. It will be used by Morgan Sindall to manage any surface water produced during the development and 2. Support any upcoming SAB planning requirements/conditions.

This document is intended to be a live and working document that should be continually updated to reflect the developing nature of the site. This will ensure that surface water risk will be managed appropriately throughout the duration of the project and the principal contractor (Morgan Sindall) should instruct the relevant party to update this plan when deemed necessary.

1.3 Information Sources

The following documents have been reviewed to inform this surface water management plan.

Supporting Documents

CIRIA C753 The SuDs Manual 2015, RP992. ISBN: 978-0-86017-760-9

Jeremy Benn Associates Limited (June 2025) - Llangatwg Community School Flood Consequences Assessment. Ref, OJR-JBAU-XX-XX-RP-Z-S3-P01-Llangatwg_School_FCA

Tetra Tech Limited (February 2025) Llangatwg Ground Investigation Report. Ref, 784-B068800

Austin-Smith: Lord (May 2025) Landscape Masterplan. Drawing no. 26CC07-ASL-30-XX-D-L-1001

Gwalia Surveyors Limited (October 2024) - Topographical Survey 3387-T-Overview

Gwalia Surveyors Limited (November 2024) - Utilities Survey 3387-U-Overview

Curtins Consultants Limited (January 2025) Llangatwg Community School – Proposed drainage Layout Sheet 1 & Sheet 2

Morgan Sindall Construction Limited (June 2025) Surface Water Management Scheme Plan.

1.4 Report Limitations

This report has been prepared with reasonable due care and diligence in accordance with industry best practice and guidance. This report has been prepared for the exclusive benefit of the client and those parties designated by them for the purpose of providing information on the measures to be undertaken during the construction phases of the development.

The conclusions presented in this report represent Ashfield's best professional judgement based upon the information available and conditions existing as of the date of this report and with due regard to the limitations of third-party data. This report scope has not included obtaining any licenses/permits/similar from Neath Port Talbot Council (NPTC) or Natural Resources Wales (NRW).

2 Site Information

2.1 Site Location

The site details are discussed in Table 1.

Table 1 – Summary of Site Details

Site Address:	Llangatwg Community School, Neath, South Wales, SA10 8DB
National Grid Reference:	276707, 199304
Site Description:	The site is currently occupied by Ysgol Gyfun Cymunedol Llangatwg (Community Comprehensive School) and the Sensitive Water Solutions Swim School. It includes associated car parking areas to the south and south-west, playgrounds located to the west and south-east and undeveloped amenity grassland used as a playing field, to the north-east.
Ground Cover:	The ground cover primarily consists of tarmacadam hardstanding, with localised areas of soft landscaping, shrubbery and mature trees situated to the south, south-west, north and east of the site. A large area of amenity grassland is located to the north-east.
Surrounding Land Uses:	To the north, the site is bordered by dense forestry, with residential dwellings, industrial, and commercial land uses located to the north-east. To the east, it is bounded by Penscynor Road, with further residential dwellings beyond. The southern boundary is defined by the A4230 roadway, beyond which lies an area of forestry followed by the A465 roadway. To the immediate west, lie residential dwellings.

2.2 Proposed Development

The proposed redevelopment includes the construction of a new school at Llangatwg, demolishing the existing school and constructing a new running track along with sports pitches. At present, Ashfield understands a planning application is being pursued with Neath Port Talbot Council. A public planning application is understood to be progressed shortly.

2.2.1 Site Phasing

The proposed development will be carried out in two phases, as shown in Figure 1. Phase 1 comprises the erection of the new buildings, whilst Phase 2 comprises the demolition of the existing school and construction of the new pitches and running track, each supported by as presented in Appendix B.

- Phase One: Located in the north-eastern portion of the site, this phase involves the construction of the new buildings within the existing amenity grassland. It includes the setup of cranes, site accommodation, fencing and hoarding to facilitate the new build.
- Phase Two: This is focused on the central area of the existing school buildings. This phase follows Phase one and involves the demolition of the existing school and the construction of a new running track and pitches.

Figure 1 – Phase Areas



2.3 Topography Analysis

A topographic survey was undertaken by Gwalia Surveyors Limited in October 2024 and is presented within Appendix B. The site generally slopes from north to south, with an average ground level of approximately 31.5mAOD. The highest point is located in the northern corner of the site, reaching 40.18mAOD with levels gradually decreasing toward the playing fields. Within the playing field area, ground levels gently fall from around 35mAOD to 31mAOD.

In the area occupied by the existing school buildings, ground levels also decline gradually in a southerly direction, ranging from approximately 33mAOD down to 30mAOD. The lowest ground levels across the site are found at the southernmost corner, specifically along the access road leading from the A4230 Main Road.

2.4 Hydrology & Hydrogeology

A Flood Consequences Assessment for the Llangatwg school was conducted by Jeremy Benn Associates Limited ("JBA")¹. This states that two Main Rivers designated by Natural Resources Wales ("NRW") are located in the vicinity of the site. The River Neath lies approximately 410 metres to the south, while the River Dulais is situated around 310 metres to the east. The Tennant Canal is located approximately 160 metres south of the site.

Within the site boundary, two unnamed ordinary watercourses are present. The first, referred to as "*unnamed ordinary watercourse 1*," flows southwest underground through the proposed phase one development area. A secondary, unnamed tributary—referred to as "*unnamed ordinary watercourse 2*"—joins watercourse 1 from the north-east. Please refer to Figures 2 and 3 for a map of watercourses and flood defences across the site².

Preliminary discussion has indicated that that an historical v-drainage ditch is present towards the north of the site in proximity to the existing sports pitches. There is an intention to re-establish this (currently known to be overgrown with vegetation and potentially obstructed with debris) and once cleared, it is anticipated that this will alleviate surface water infiltration from the high-level off site topography.

Based upon the received Tetra Tech Phase 2 Ground Investigation Report³, the site is underlain by Made ground, glaciofluvial deposits comprising Sand and Gravel and the Brithdir Member comprised of sandstone. The Brithdir member and superficial deposits are designated as a Secondary A aquifer.

The ground conditions at the site consist of Topsoil between 0.0-1.0m below ground level (bgl), underlain by occasional Made Ground approximately 0.90m thick, which is further underlain by Glaciofluvial Sheet Deposits. A total of three infiltration tests were carried out within the weathered bedrock, Made Ground and Superficial Deposits, all of which failed.

The site is not situated within a groundwater Source Protection Zone (SPZ).

¹ Jeremy Benn Associates Limited (June 2025) - Llangatwg Community School Flood Consequences Assessment. Ref, OJR-JBAU-XX-XX-RP-Z-S3-P01-Llangatwg_School_FCA

² JBA Consulting Llangatwg Community School Flood Consequences Assessment

³ Tetra Tech Limited Llangatwg Ground Investigation Report (February 2025)

Figure 2 Watercourses and Flood Defences

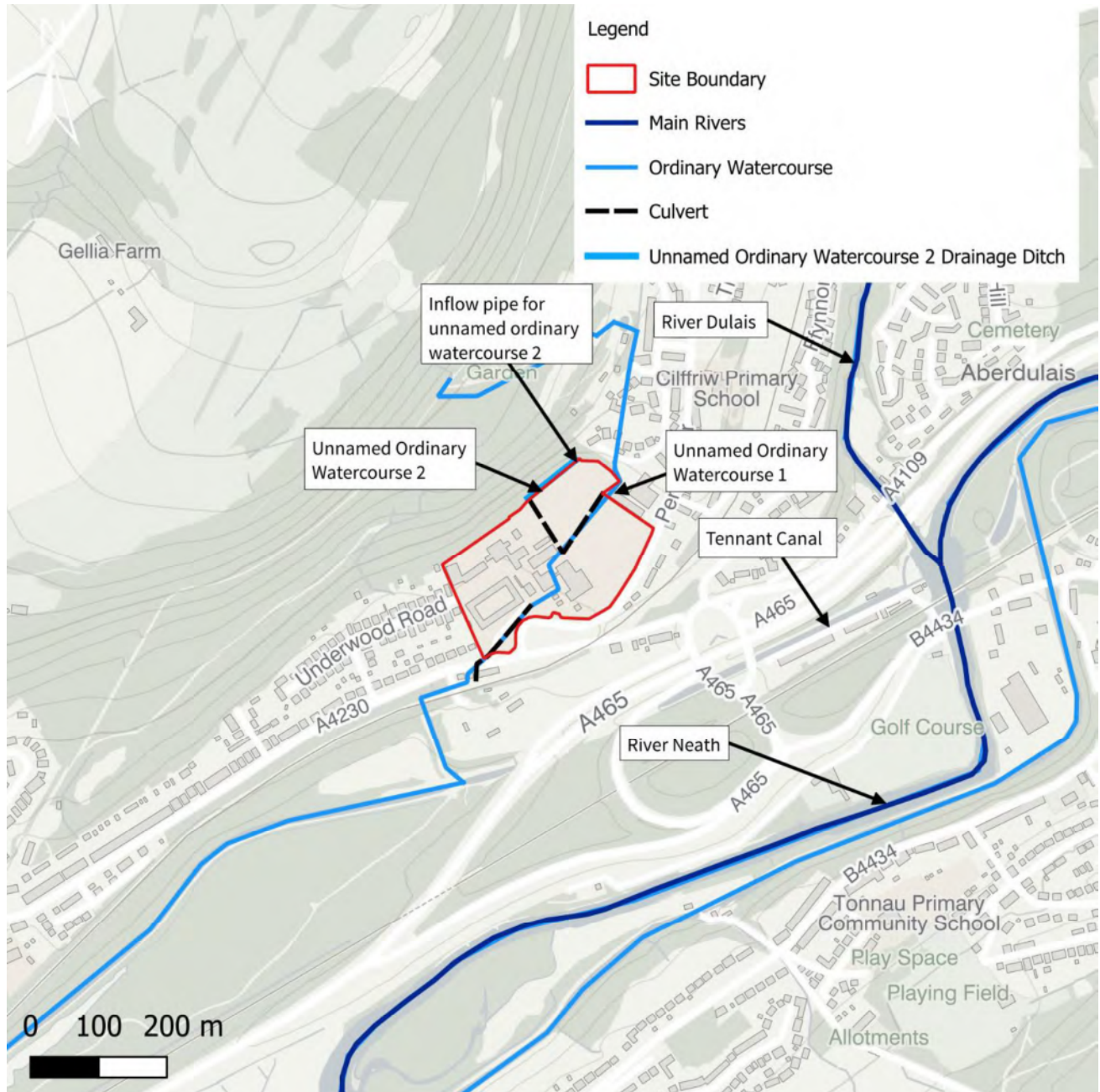
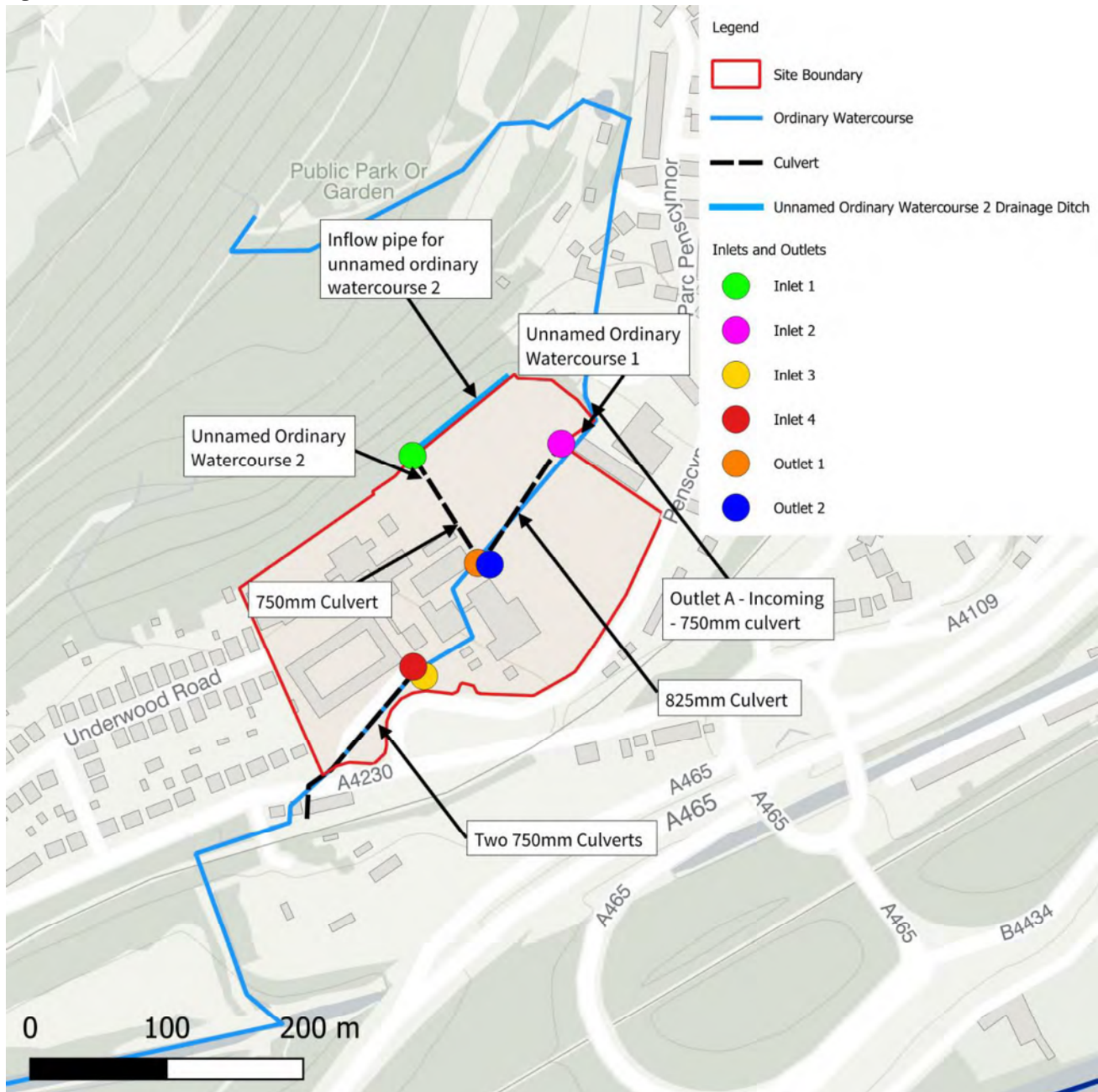


Figure 3 Watercourses at the Site



2.5 Existing Drainage

The current drainage system at and around Llangatwg Community School consists of separate foul, surface water, and combined sewer networks. Foul drainage primarily runs through developed areas, connecting to inspection chambers and outfalls. Surface water drainage is mainly located toward the northern part of the site. The combined network indicates sections where both foul and surface water are conveyed through the same pipes. For a plan of existing drainage, please refer to Appendix D.

2.6 Surface Water Drainage

To support flood mitigation, a series of Sustainable Drainage Systems (“SuDS”), including swales and attenuation basins, will be implemented on site. These features are designed to accommodate and store significant volumes of surface water entering the area. Their proposed locations and design are illustrated in the Landscape Masterplan, provided in Appendix E.

3 Run-off Rate

3.1 Phase 1 Run-off Rate

The QBAR (the mean annual flood event (1 in 2.3 year storm)) run-off rate for the Phase 1 catchment area (2.42ha) has been estimated using the ICP SUDS calculation within InfoDrainage. This has been calculated to be 15.3l/s. The calculations and results are included within Figure 4.

Figure 4 – ICP SUDS Calculations

ICP SUDS / IH 124	ADAS 345	FEH	ReFH2	Greenfield Volume												
Method ☒ ICP SUDS ☐ IH 124 Area (ha) 2.42 SAAR (mm) 1373.0 Map Soil 0.370 Region Region 9 ... Additional Options Urban 0.000 Return Period (years) 0 Growth Curve (None) Calculate																
Results <table border="1"> <thead> <tr> <th>Region</th> <th>QBAR Rural (L/s)</th> <th>QBAR Urban (L/s)</th> <th>Q 1 (years) (L/s)</th> <th>Q 30 (years) (L/s)</th> <th>Q 100 (years) (L/s)</th> </tr> </thead> <tbody> <tr> <td>Region 9</td> <td>15.3</td> <td>15.3</td> <td>13.5</td> <td>27.0</td> <td>33.3</td> </tr> </tbody> </table>					Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)	Region 9	15.3	15.3	13.5	27.0	33.3
Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)											
Region 9	15.3	15.3	13.5	27.0	33.3											

A rate of 15.3l/s will be used to calculate the volume of storage that would need to be provided by the water treatment system on site (discussed further in Section 8).

3.2 Phase 2 Run-off Rate

The QBAR run-off rate for the Phase 2 catchment area (2.23ha), which has been estimated using the ICP SUDS calculation within InfoDrainage, has been calculated to be 14.1l/s. The calculations and results are included within Figure 5.

Figure 5 – ICP SUDS Calculations

ICP SUDS / IH 124	ADAS 345	FEH	ReFH2	Greenfield Volume												
Method ☒ ICP SUDS ☐ IH 124 Area (ha) 2.23 SAAR (mm) 1373.0 Map Soil 0.370 Region Region 9 ... Additional Options Urban 0.000 Return Period (years) 0 Growth Curve (None) Calculate																
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Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)											
Region 9	14.1	14.1	12.4	24.8	30.7											

A rate of 14.1l/s will be used to calculate the volume of storage that would need to be provided by the water treatment system on site (discussed further in Section 8).

4 Construction Phasing

As discussed in Section 2.2.1, the development will be delivered in two phases. A high-level summary of the phasing is provided in Appendix B.

1. Enabling Works:

- Site accommodation units installed in the northeast corner for welfare and office use.
- Setup of site fencing.
- Initial access routes created via the south entrance, including setup of haul roads.
- Clearance of the existing v-drainage ditch to the north.
- Temporary contractor car park set up adjacent to the welfare area and access point.
- Site clearance and tree removal in preparation for the new build footprint.

2. Phase 1 - Construction Works

- Construction of the new school building to the east of the site.
- Construction of the new 3G youth football/rugby 9v9 pitch to the northeast.
- Construct southern car parking area

3. Phase 2 Demolition Phase

- Demolition of existing school once the new build is occupied.
- Construction of new pitches and running track following demolition.

4.1 Proposed Drainage

The proposed construction phase drainage has been set out to manage surface water within the site boundary. The site is divided into multiple drainage catchments. Surface water is to be directed via a combination of linear drainage channels, gullies and manholes into the underlying pipe network that connects to attenuation features (basin feature, swale), which the majority then outfall into the existing culvert located towards the centre of the site. Please refer to Appendix G for the drainage plans produced by Curtins Consulting Limited.

4.1.1 Foul Drainage

Foul drainage will be routed around the new school building. All internal foul drainage pipes will be encased in concrete, with a minimum internal diameter of 100mm and laid to a hydraulic gradient of 1 in 40. The proposed foul drainage system will discharge into the existing DCWW combined sewer towards the east of the site, subject to approval via a Section 106 Agreement.

5 Roles & Responsibilities

In order for the SWMP to be successfully implemented, that key roles and responsibilities for waste management be clearly defined, documented and communicated. The roles and responsibilities with regards to the SWMP during the project are shown in table below. The main contact for the site is Mr Alex Smith who can be contacted on 07866146782 or alex.smith@morgansindall.com. The contact details of the roles shown below are to be confirmed and subject to change.

Table 3 - Key Project Members

Name	Role and Responsibility	Contact Number	Email Address
Alex Smith	Project Director (PD) Undertake both an on-site and off-site role, ensuring that all appointed managers and other positions undertake their duties and achieve the required outcome of the project. Provide guidance and direction to the project team.	07866 146782	Alex.Smith@morgansindall.com
Paul Hugill	Health and Safety Manager (HS) Assist both the office and site-based teams to meeting their legal duties under the Construction (Design and Management) Regulations 2015 and all other relevant Acts, Regulations, Approved Codes or Practise and Industry Specifications, providing competent Health and Safety Advice for the Project Management Team, as and when required, and undertaking audits of the project to monitor site performance.	TBC by Morgan Sindall	TBC by Morgan Sindall
Mark Stephens	Operations Manager (OM) Based off site, planning for and overseeing the project, ensuring all members of the project team prepare for and deliver the project to the highest standards. Liaising with the project team to ensure that all specification / issues are either delivered or resolved.	TBC by Morgan Sindall	TBC by Morgan Sindall
Alex Smith	Project Manager (PM) Based both on and off site. Liaises with the project team to ensure that communication of site issues is correctly managed and delivered.	07866 146782	Alex.Smith@morgansindall.com
Alex Smith	Project Engineer/Supervisor Supervises/carries out sampling/testing of samples and site conditions through the drilling works. Responsible for system operation and site monitoring.	07866 146782	Alex.Smith@morgansindall.com

6 Hazard Risk Assessment & Controls

Excavations as part of the foundations, drainage works, and general earthworks may result in the generation of silt laden water which will require removal and discharge. As a result, it can be assumed that the groundwater encountered in excavations, equipment washdown, and run-off may be contaminated with silt and will require control measures prior to its discharge.

The table below outlines the hazards/risk associated with the construction phase and the control measures needed to mitigate their impact.

Table 4 - Summary Hazard and Control Measures

Hazard/Risk	Control Measure
Surface Water Run-off/Overland Flow The site will be excavated and filled to facilitate the new development. This process exposes the topsoil making it susceptible to surface water run-off and erosion, which can result in silt entering adjacent properties of surface water features.	- Exposed soil to be sealed to minimise silty run-off. - Contoured berms to be installed to prevent whole area run-off and prevent erosion. - Geotextile barriers (silt barriers) to be used to stop the migration of silts and surface water run-off.
Excavations & Dewatering Surface water or groundwater may enter the excavation as locations are advanced. This water will require removal to allow safe excavation and facilitate the installation of foundations or services. During dewatering, the water can pick up sediment and suspended soils from the excavation site, carrying them in surface water.	- Excavation depth and area to be minimised to prevent large volumes of groundwater ingress. - Excavation will not be left for significant periods of time. - Excavation edges will have bunded edges to prevent overland flow. - Pumped water will be treated and undertake silt separation measures prior to discharge. Methods of water treatment are discussed in Section 6 of this report.
Welfare Unit (if present) Units are likely to contain soap and bacteria arising from the washing of hands which may contain contaminants associated with the site works (e.g. diesel).	- Locate welfare unit away from the know surface water features. - Ensure all waste from the welfare unit, including sewage and greywater, is collected and disposed of properly through a licensed waste management service. - Use sealed tanks for sewage and greywater to prevent leaks and spills. - Install bunds or secondary containment around welfare units to catch any potential spills or leaks if at high risk. - Use silt fences, sediment traps, or other erosion control measures to manage surface water run-off and prevent it from carrying contaminants into nearby water bodies. - Remove significant contamination with specialist wipes before washing hands with water. - Position welfare unit on hard standing away from drainage gullies where possible. Use of neoprene drainage mats. - Welfare unit to be serviced weekly.
Refuelling	Establish specific refuelling zones away from any know surface water features.

Hazard/Risk	Control Measure
Water within drip trays/bunds contaminated with hydrocarbons.	■ Use containment areas with impermeable surfaces to prevent fuel seepage into the ground. ■ Install secondary containment systems such as bunds or spill trays around refuelling areas to catch any fuel spills. ■ Use spill kits containing absorbent materials, booms, and other tools to manage spills promptly. ■ Store fuel in approved, double-walled tanks that are resistant to leaks and equipped with spill containment features. ■ Excess wastewater generated during drilling will be pumped directly into a bunded Intermediate Bulk Container (IBC).
Haul Road Run-off Wastewater generated from rainfall/surface water run-off.	■ Program the installation of a solid haulage roads early in the project program. ■ Design haul roads with proper grading to direct run-off towards controlled drainage systems. ■ Minimise the site haul roads length. ■ Use gravel or crushed stone to surface haul roads, which reduces dust, erosion, and sediment run-off. ■ Establish vegetative cover along the sides of haul roads to stabilize soil and reduce erosion. Use fast-growing grasses or ground covers. ■ Install silt fences along the perimeter of haul roads to trap sediment and prevent it from leaving the site. ■ Use sediment traps to capture and retain sediments from road run-off.
Wheel Wash Provides a source of silt water which has the potential to cause issues within local watercourses. Wheel wash may also be a source of contaminants arising from the vehicles that pass over the unit in terms of diesel or engine oil leaks.	■ Install the wheel wash station away from watercourses, drainage systems, and environmentally sensitive areas. ■ Design the wheel wash area with impermeable surfaces and containment systems to capture and contain wash water. ■ Ensure the area is graded to direct all wash water to a containment system or treatment facility. ■ Filling or top up of the wash tank to be level controlled. ■ Routine maintenance undertaken including silt removal. ■ Spray reduction measures to be in place to reduce water loss. ■ Unit to be equipped with a sediment trap. ■ Require all vehicles to pass through the wheel wash station before exiting the site to minimize soil and sediment tracking.
Wash Down Areas Required when site equipment and plant become coated in mud or contamination and will require washing down prior to moving positions to avoid cross contamination.	■ Locate washdown of plant/equipment away from the culvert. ■ Area to be bunded for redirecting of wastewater away from known surface water features. ■ Drains to be covered using neoprene drain mats and/or spill booms.
Run-off from Concreting Can occur when water comes in contact with uncured or freshly poured concrete.	■ Employ sediment traps and silt barriers within the concrete washout areas to prevent untreated water entering known surface water features.

Hazard/Risk	Control Measure
	■ Treat water prior to discharge to remove sediment.

6.1 Site Personnel Control Measures

The following measures are considered to represent good practice⁴ to increase awareness and ensure that surface water management activities are sufficiently documented and maintained throughout the works.

- All groundworker subcontractors should ideally consider minimisation of silt and its management within their Risk Assessment Method Statement ("RAMS") for their work.
- Specific site-based awareness training (or Toolbox Talks) on the management of surface water and silt activities could be provided for groundwork subcontractors.
- Site induction folder ideally should include items relating to silt management and surface water bund protection measures so they can be maintained throughout the works.
- Surface water checks could be undertaken weekly to ensure that risk items are identified early on and managed prior to potential incidents (gully monitoring, potential receptors, maintenance of drainage/treatment systems).

6.2 Dust suppression

Dust suppression measures will be used in dry conditions in order to prevent dust from being carried into watercourses and other sensitive neighbouring habitats/residential properties. Dust suppression measures should be clearly communicated to all contractors involving strictly enforcing on-Site speed limits, dampening down the road at regular intervals by spraying water, scraping dust collected on roads and sealing stockpile materials. The frequency of the suppression measures (excluding the speed limit) will be varied dependant on the weather conditions and ground conditions within the working areas.

⁴ DEFRA (March 2010) Surface Water Management Plan Technical Guidance.

7 Dewatering of Excavations

The aim of dewatering will be to remove both groundwater ingress and surface water from areas of excavation. The strategy involves deploying a pumping system in the excavated areas, with pumped water being directed to one of the three designated sediment basins located on site (refer to Appendix B). From here, water will be treated (refer to Section 8.0 for treatment options) with sediment filtration before discharge into the existing V-ditch which runs across the northern boundary of the site. The V-ditch is to be cleaned and reinstated as part of the enabling works. Ultimately, water from this area will then discharge into the existing culvert present on site via an Ordinary Watercourse Consent ("OWC") which is anticipated to be granted by Neath Port Talbot Council.

In addition to the sediment basins and V-ditch system, water may also be directed through a network of French drains and the existing storm culvert. These form part of the temporary drainage infrastructure to manage both surface and groundwater on-site.

A new foul and rising main has been proposed to be installed on site to act as a back-up in if an appropriate OWC permit cannot be obtained from the LPA. However, this will require approval from Dŵr Cymru Welsh Water ("DCWW").

Dewatering will continue as required during Phase 2, utilising the same temporary infrastructure, including the sediment basins and associated infiltration via the V-ditch. Dewatering in Phase 2 is understood to be dealt with by the site team.

Please refer to Appendix H for a Phase 1 De-Watering Strategy.

7.1 Surface Water Permitting Considerations

It is understood following consultation that DCWW are not currently allowing surface water flows to discharge into any combined sewer networks. It is also a requirement that any discharge containing silt or contaminants should be treated before release with the aim of reducing pollution and managing flows at the highest priority levels possible. DCWW operate the following hierarchy approach:

- Priority Level 1: Water run-off is collected for use.
- Priority Level 2: Water run-off is infiltrated to the ground.
- Priority Level 3: Water run-off is discharged to a surface water body.
- Priority Level 4: Water run-off is discharged to a surface water sewer; highway drain or another drainage system.
- Priority Level 5: Water run-off is discharged to a combined sewer.

Based on review of applicable guidance and consultation with Neath Port Talbot Council SABS Team, an Ordinary Watercourse Consent ("OWC") into the existing surface water culvert appears to be the most appropriate approach for managing the volumes of surface water anticipated during heavy rainfall events.

If neither RPS261 nor an OWC/NRW temporary discharge permit is available, the proposed foul and rising main may be utilised as a contingency, subject to prior consultation and formal approval from DCWW. Discharge to the combined sewer system is the least preferred option within the DCWW drainage hierarchy and should only be pursued once all other options have been exhausted.