



Cwm Planning & Design Ltd.
(Civil, Structural & Architectural)

STRUCTURAL CALCULATIONS

NEW DWELLING – REAR GARDEN RETAINING WALLS

at:

**54 Lucy Road
Skewen
Neath**

Date: 23rd January 2024

BRIEF:

- 1) The design of a Reinforced Concrete retaining wall to form a rear wall to a proposed garage and support the front wall of a proposed New residential dwelling.

REFERENCES:

Architectural Drawings
SCALE structural design package (for unit loads)
TEDDS structural design computer package.

DESIGN LOADINGS:

Refer to structural calculations attached.

Surcharge (Garden) = **1.5kN**

SUMMARY

Retained height taken as **2.7m (2.4m clear)** (300mm cover to front)
See Section Drawing for details

RETAINING WALL
DESIGN CALCULATIONS

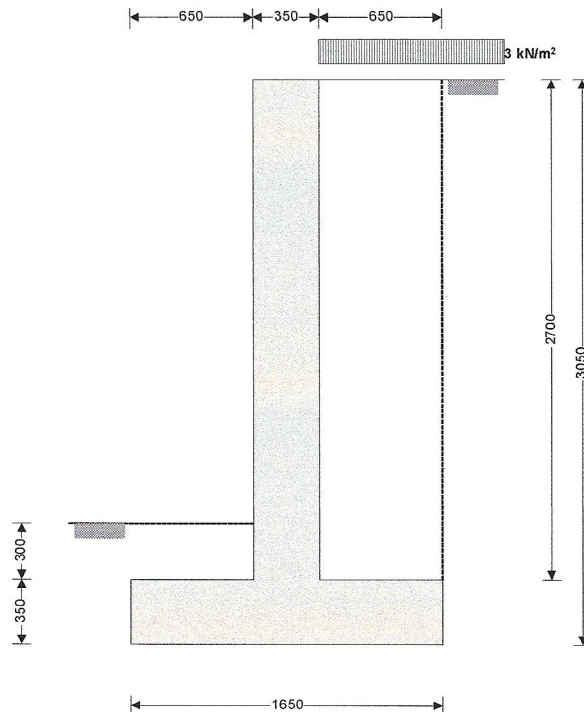


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RETAINING WALL ANALYSIS (BS 8002:1994)

TEDDS calculation version 1.2.01.08



Wall details

Retaining wall type
Height of retaining wall stem
Thickness of wall stem
Length of toe
Length of heel
Overall length of base
Thickness of base
Depth of downstand
Position of downstand
Thickness of downstand
Height of retaining wall
Depth of cover in front of wall
Depth of unplanned excavation
Height of ground water behind wall
Height of saturated fill above base
Density of wall construction
Density of base construction
Angle of rear face of wall
Angle of soil surface behind wall
Effective height at virtual back of wall

Unpropped cantilever

$h_{\text{stem}} = 2700 \text{ mm}$
 $t_{\text{wall}} = 350 \text{ mm}$
 $l_{\text{toe}} = 650 \text{ mm}$
 $l_{\text{heel}} = 650 \text{ mm}$
 $l_{\text{base}} = l_{\text{toe}} + l_{\text{heel}} + t_{\text{wall}} = 1650 \text{ mm}$
 $t_{\text{base}} = 350 \text{ mm}$
 $d_{\text{ds}} = 0 \text{ mm}$
 $l_{\text{ds}} = 0 \text{ mm}$
 $t_{\text{ds}} = 350 \text{ mm}$
 $h_{\text{wall}} = h_{\text{stem}} + t_{\text{base}} + d_{\text{ds}} = 3050 \text{ mm}$
 $d_{\text{cover}} = 300 \text{ mm}$
 $d_{\text{exc}} = 0 \text{ mm}$
 $h_{\text{water}} = 0 \text{ mm}$
 $h_{\text{sat}} = \max(h_{\text{water}} - t_{\text{base}} - d_{\text{ds}}, 0 \text{ mm}) = 0 \text{ mm}$
 $\gamma_{\text{wall}} = 23.6 \text{ kN/m}^3$
 $\gamma_{\text{base}} = 23.6 \text{ kN/m}^3$
 $\alpha = 90.0 \text{ deg}$
 $\beta = 0.0 \text{ deg}$
 $h_{\text{eff}} = h_{\text{wall}} + l_{\text{heel}} \times \tan(\beta) = 3050 \text{ mm}$

Retained material details

Mobilisation factor $M = 1.5$



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Moist density of retained material

$$\gamma_m = 18.0 \text{ kN/m}^3$$

Saturated density of retained material

$$\gamma_s = 21.0 \text{ kN/m}^3$$

Design shear strength

$$\phi' = 24.2 \text{ deg}$$

Angle of wall friction

$$\delta = 18.6 \text{ deg}$$

Base material details

Stiff or hard glacial clay

Moist density

$$\gamma_{mb} = 18.0 \text{ kN/m}^3$$

Design shear strength

$$\phi'_b = 24.2 \text{ deg}$$

Design base friction

$$\delta_b = 18.6 \text{ deg}$$

Allowable bearing pressure

$$P_{\text{bearing}} = 150 \text{ kN/m}^2$$

Using Coulomb theory

Active pressure coefficient for retained material

$$K_a = \sin(\alpha + \phi')^2 / (\sin(\alpha)^2 \times \sin(\alpha - \delta) \times [1 + \sqrt{(\sin(\phi' + \delta) \times \sin(\phi' - \beta) / (\sin(\alpha - \delta) \times \sin(\alpha + \beta)))}]^2) = 0.369$$

Passive pressure coefficient for base material

$$K_p = \sin(90 - \phi'_b)^2 / (\sin(90 - \delta_b) \times [1 - \sqrt{(\sin(\phi'_b + \delta_b) \times \sin(\phi'_b) / (\sin(90 + \delta_b)))}]^2) = 4.187$$

At-rest pressure

At-rest pressure for retained material

$$K_0 = 1 - \sin(\phi') = 0.590$$

Loading details

Surcharge load on plan

$$\text{Surcharge} = 2.5 \text{ kN/m}^2$$

Applied vertical dead load on wall

$$W_{\text{dead}} = 0.0 \text{ kN/m}$$

Applied vertical live load on wall

$$W_{\text{live}} = 0.0 \text{ kN/m}$$

Position of applied vertical load on wall

$$l_{\text{load}} = 0 \text{ mm}$$

Applied horizontal dead load on wall

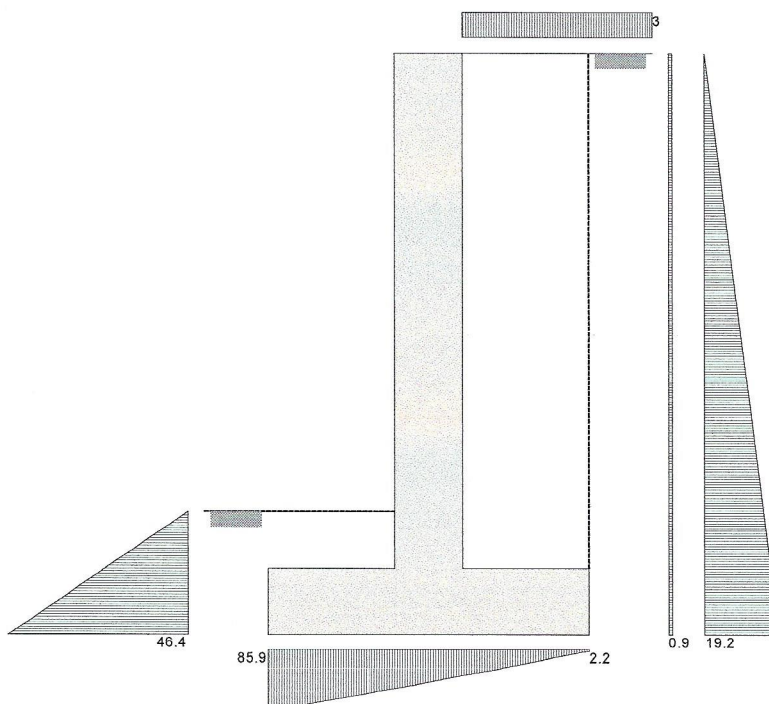
$$F_{\text{dead}} = 0.0 \text{ kN/m}$$

Applied horizontal live load on wall

$$F_{\text{live}} = 0.0 \text{ kN/m}$$

Height of applied horizontal load on wall

$$h_{\text{load}} = 0 \text{ mm}$$



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Loads shown in kN/m, pressures shown in kN/m²

Vertical forces on wall

Wall stem	$w_{wall} = h_{stem} \times t_{wall} \times \gamma_{wall} = 22.3 \text{ kN/m}$
Wall base	$w_{base} = l_{base} \times t_{base} \times \gamma_{base} = 13.6 \text{ kN/m}$
Surcharge	$w_{sur} = \text{Surcharge} \times l_{heel} = 1.6 \text{ kN/m}$
Moist backfill to top of wall	$w_{m_w} = l_{heel} \times (h_{stem} - h_{sat}) \times \gamma_m = 31.6 \text{ kN/m}$
Soil in front of wall	$w_p = l_{toe} \times d_{cover} \times \gamma_{mb} = 3.5 \text{ kN/m}$
Total vertical load	$W_{total} = w_{wall} + w_{base} + w_{sur} + w_{m_w} + w_p = 72.7 \text{ kN/m}$

Horizontal forces on wall

Surcharge	$F_{sur} = K_a \times \cos(90 - \alpha + \delta) \times \text{Surcharge} \times h_{eff} = 2.7 \text{ kN/m}$
Moist backfill above water table	$F_{m_a} = 0.5 \times K_a \times \cos(90 - \alpha + \delta) \times \gamma_m \times (h_{eff} - h_{water})^2 = 29.3 \text{ kN/m}$
Total horizontal load	$F_{total} = F_{sur} + F_{m_a} = 32 \text{ kN/m}$

Calculate stability against sliding

Passive resistance of soil in front of wall	$F_p = 0.5 \times K_p \times \cos(\delta_b) \times (d_{cover} + t_{base} + d_{ds} - d_{exc})^2 \times \gamma_{mb} = 15.1 \text{ kN/m}$
Resistance to sliding	$F_{res} = F_p + (W_{total} - w_{sur} - w_p) \times \tan(\delta_b) = 37.8 \text{ kN/m}$

PASS - Resistance force is greater than sliding force

Overturning moments

Surcharge	$M_{sur} = F_{sur} \times (h_{eff} - 2 \times d_{ds}) / 2 = 4.1 \text{ kNm/m}$
Moist backfill above water table	$M_{m_a} = F_{m_a} \times (h_{eff} + 2 \times h_{water} - 3 \times d_{ds}) / 3 = 29.8 \text{ kNm/m}$
Total overturning moment	$M_{ot} = M_{sur} + M_{m_a} = 33.8 \text{ kNm/m}$

Restoring moments

Wall stem	$M_{wall} = w_{wall} \times (l_{toe} + t_{wall} / 2) = 18.4 \text{ kNm/m}$
Wall base	$M_{base} = w_{base} \times l_{base} / 2 = 11.2 \text{ kNm/m}$
Moist backfill	$M_{m_r} = (w_{m_w} \times (l_{base} - l_{heel} / 2) + w_{m_s} \times (l_{base} - l_{heel} / 3)) = 41.9 \text{ kNm/m}$
Total restoring moment	$M_{rest} = M_{wall} + M_{base} + M_{m_r} = 71.5 \text{ kNm/m}$

Check stability against overturning

Total overturning moment	$M_{ot} = 33.8 \text{ kNm/m}$
Total restoring moment	$M_{rest} = 71.5 \text{ kNm/m}$

PASS - Restoring moment is greater than overturning moment

Check bearing pressure

Surcharge	$M_{sur_r} = w_{sur} \times (l_{base} - l_{heel} / 2) = 2.2 \text{ kNm/m}$
Soil in front of wall	$M_{p_r} = w_p \times l_{toe} / 2 = 1.1 \text{ kNm/m}$
Total moment for bearing	$M_{total} = M_{rest} - M_{ot} + M_{sur_r} + M_{p_r} = 40.9 \text{ kNm/m}$
Total vertical reaction	$R = W_{total} = 72.7 \text{ kN/m}$
Distance to reaction	$x_{bar} = M_{total} / R = 564 \text{ mm}$
Eccentricity of reaction	$e = \text{abs}((l_{base} / 2) - x_{bar}) = 261 \text{ mm}$

Reaction acts within middle third of base

Bearing pressure at toe	$p_{toe} = (R / l_{base}) + (6 \times R \times e / l_{base}^2) = 85.9 \text{ kN/m}^2$
Bearing pressure at heel	$p_{heel} = (R / l_{base}) - (6 \times R \times e / l_{base}^2) = 2.2 \text{ kN/m}^2$

PASS - Maximum bearing pressure is less than allowable bearing pressure

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RETAINING WALL DESIGN (BS 8002:1994)

TEDDS calculation version 1.2.01.08

Ultimate limit state load factors

Dead load factor	$\gamma_{f_d} = 1.4$
Live load factor	$\gamma_{f_l} = 1.6$
Earth and water pressure factor	$\gamma_{f_e} = 1.4$

Factored vertical forces on wall

Wall stem	$W_{wall_f} = \gamma_{f_d} \times h_{stem} \times t_{wall} \times \gamma_{wall} = 31.2 \text{ kN/m}$
Wall base	$W_{base_f} = \gamma_{f_d} \times l_{base} \times t_{base} \times \gamma_{base} = 19.1 \text{ kN/m}$
Surcharge	$W_{sur_f} = \gamma_{f_l} \times \text{Surcharge} \times l_{heel} = 2.6 \text{ kN/m}$
Moist backfill to top of wall	$W_{m_w_f} = \gamma_{f_d} \times l_{heel} \times (h_{stem} - h_{sat}) \times \gamma_m = 44.2 \text{ kN/m}$
Soil in front of wall	$W_{p_f} = \gamma_{f_d} \times l_{toe} \times d_{cover} \times \gamma_{mb} = 4.9 \text{ kN/m}$
Total vertical load	$W_{total_f} = W_{wall_f} + W_{base_f} + W_{sur_f} + W_{m_w_f} + W_{p_f} = 102 \text{ kN/m}$

Factored horizontal at-rest forces on wall

Surcharge	$F_{sur_f} = \gamma_{f_l} \times K_0 \times \text{Surcharge} \times h_{eff} = 7.2 \text{ kN/m}$
Moist backfill above water table	$F_{m_a_f} = \gamma_{f_e} \times 0.5 \times K_0 \times \gamma_m \times (h_{eff} - h_{water})^2 = 69.2 \text{ kN/m}$
Total horizontal load	$F_{total_f} = F_{sur_f} + F_{m_a_f} = 76.4 \text{ kN/m}$
Passive resistance of soil in front of wall	$F_{p_f} = \gamma_{f_e} \times 0.5 \times K_p \times \cos(\delta_b) \times (d_{cover} + t_{base} + d_{ds} - d_{exc})^2 \times \gamma_{mb} = 21.1 \text{ kN/m}$

Factored overturning moments

Surcharge	$M_{sur_f} = F_{sur_f} \times (h_{eff} - 2 \times d_{ds}) / 2 = 11 \text{ kNm/m}$
Moist backfill above water table	$M_{m_a_f} = F_{m_a_f} \times (h_{eff} + 2 \times h_{water} - 3 \times d_{ds}) / 3 = 70.3 \text{ kNm/m}$
Total overturning moment	$M_{ot_f} = M_{sur_f} + M_{m_a_f} = 81.3 \text{ kNm/m}$

Restoring moments

Wall stem	$M_{wall_f} = W_{wall_f} \times (l_{toe} + t_{wall} / 2) = 25.8 \text{ kNm/m}$
Wall base	$M_{base_f} = W_{base_f} \times l_{base} / 2 = 15.7 \text{ kNm/m}$
Surcharge	$M_{sur_r_f} = W_{sur_f} \times (l_{base} - l_{heel} / 2) = 3.4 \text{ kNm/m}$
Moist backfill	$M_{m_r_f} = (W_{m_w_f} \times (l_{base} - l_{heel} / 2) + W_{m_s_f} \times (l_{base} - l_{heel} / 3)) = 58.6 \text{ kNm/m}$
Soil in front of wall	$M_{p_r_f} = W_{p_f} \times l_{toe} / 2 = 1.6 \text{ kNm/m}$
Total restoring moment	$M_{rest_f} = M_{wall_f} + M_{base_f} + M_{sur_r_f} + M_{m_r_f} + M_{p_r_f} = 105.1 \text{ kNm/m}$

Factored bearing pressure

Total moment for bearing	$M_{total_f} = M_{rest_f} - M_{ot_f} = 23.8 \text{ kNm/m}$
Total vertical reaction	$R_r = W_{total_f} = 102.0 \text{ kN/m}$
Distance to reaction	$x_{bar_f} = M_{total_f} / R_r = 234 \text{ mm}$
Eccentricity of reaction	$e_f = \text{abs}((l_{base} / 2) - x_{bar_f}) = 591 \text{ mm}$
Reaction acts outside middle third of base	
Bearing pressure at toe	$p_{toe_f} = R_r / (1.5 \times x_{bar_f}) = 291.1 \text{ kN/m}^2$
Bearing pressure at heel	$p_{heel_f} = 0 \text{ kN/m}^2 = 0 \text{ kN/m}^2$
Rate of change of base reaction	$\text{rate} = p_{toe_f} / (3 \times x_{bar_f}) = 415.22 \text{ kN/m}^2/\text{m}$
Bearing pressure at stem / toe	$p_{stem_toe_f} = \text{max}(p_{toe_f} - (\text{rate} \times l_{toe}), 0 \text{ kN/m}^2) = 21.2 \text{ kN/m}^2$
Bearing pressure at mid stem	$p_{stem_mid_f} = \text{max}(p_{toe_f} - (\text{rate} \times (l_{toe} + t_{wall} / 2)), 0 \text{ kN/m}^2) = 0 \text{ kN/m}^2$
Bearing pressure at stem / heel	$p_{stem_heel_f} = \text{max}(p_{toe_f} - (\text{rate} \times (l_{toe} + t_{wall})), 0 \text{ kN/m}^2) = 0 \text{ kN/m}^2$

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Design of reinforced concrete retaining wall toe (BS 8002:1994)

Material properties

Characteristic strength of concrete $f_{cu} = 35 \text{ N/mm}^2$
 Characteristic strength of reinforcement $f_y = 500 \text{ N/mm}^2$

Base details

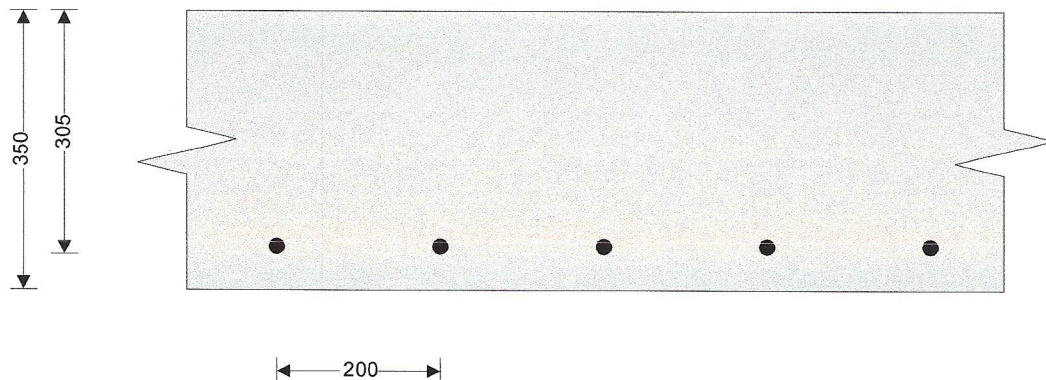
Minimum area of reinforcement $k = 0.13 \%$
 Cover to reinforcement in toe $c_{toe} = 35 \text{ mm}$

Calculate shear for toe design

Shear from bearing pressure $V_{toe_bear} = (p_{toe_f} + p_{stem_toe_f}) \times l_{toe} / 2 = 101.5 \text{ kN/m}$
 Shear from weight of base $V_{toe_wt_base} = \gamma_{f_d} \times \gamma_{base} \times l_{toe} \times t_{base} = 7.5 \text{ kN/m}$
 Shear from weight of soil $V_{toe_wt_soil} = w_{p_f} - (\gamma_{f_d} \times \gamma_m \times l_{toe} \times d_{exc}) = 4.9 \text{ kN/m}$
 Total shear for toe design $V_{toe} = V_{toe_bear} - V_{toe_wt_base} - V_{toe_wt_soil} = 89.1 \text{ kN/m}$

Calculate moment for toe design

Moment from bearing pressure $M_{toe_bear} = (2 \times p_{toe_f} + p_{stem_mid_f}) \times (l_{toe} + t_{wall} / 2)^2 / 6 = 66 \text{ kNm/m}$
 Moment from weight of base $M_{toe_wt_base} = (\gamma_{f_d} \times \gamma_{base} \times t_{base} \times (l_{toe} + t_{wall} / 2)^2 / 2) = 3.9 \text{ kNm/m}$
 Moment from weight of soil $M_{toe_wt_soil} = (w_{p_f} - (\gamma_{f_d} \times \gamma_m \times l_{toe} \times d_{exc})) \times (l_{toe} + t_{wall}) / 2 = 2.5 \text{ kNm/m}$
 Total moment for toe design $M_{toe} = M_{toe_bear} - M_{toe_wt_base} - M_{toe_wt_soil} = 59.7 \text{ kNm/m}$



Check toe in bending

Width of toe $b = 1000 \text{ mm/m}$
 Depth of reinforcement $d_{toe} = t_{base} - c_{toe} - (\phi_{toe} / 2) = 305.0 \text{ mm}$
 Constant $K_{toe} = M_{toe} / (b \times d_{toe}^2 \times f_{cu}) = 0.018$

Compression reinforcement is not required

Lever arm $z_{toe} = \min(0.5 + \sqrt{(0.25 - (\min(K_{toe}, 0.225) / 0.9))}, 0.95) \times d_{toe}$
 $z_{toe} = 290 \text{ mm}$

Area of tension reinforcement required $A_{s_toe_des} = M_{toe} / (0.87 \times f_y \times z_{toe}) = 473 \text{ mm}^2/\text{m}$
 Minimum area of tension reinforcement $A_{s_toe_min} = k \times b \times t_{base} = 455 \text{ mm}^2/\text{m}$
 Area of tension reinforcement required $A_{s_toe_req} = \text{Max}(A_{s_toe_des}, A_{s_toe_min}) = 473 \text{ mm}^2/\text{m}$
 Reinforcement provided **20 mm dia.bars @ 200 mm centres**
 Area of reinforcement provided $A_{s_toe_prov} = 1571 \text{ mm}^2/\text{m}$

PASS - Reinforcement provided at the retaining wall toe is adequate

Check shear resistance at toe

Design shear stress $v_{toe} = V_{toe} / (b \times d_{toe}) = 0.292 \text{ N/mm}^2$
 Allowable shear stress $v_{adm} = \min(0.8 \times \sqrt{f_{cu}} / 1 \text{ N/mm}^2, 5) \times 1 \text{ N/mm}^2 = 4.733 \text{ N/mm}^2$

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PASS - Design shear stress is less than maximum shear stress

From BS8110:Part 1:1997 – Table 3.8

Design concrete shear stress

$$v_{c_toe} = 0.606 \text{ N/mm}^2$$

$v_{toe} < v_{c_toe}$ - No shear reinforcement required

Design of reinforced concrete retaining wall heel (BS 8002:1994)

Material properties

Characteristic strength of concrete

$$f_{cu} = 35 \text{ N/mm}^2$$

Characteristic strength of reinforcement

$$f_y = 500 \text{ N/mm}^2$$

Base details

Minimum area of reinforcement

$$k = 0.13 \%$$

Cover to reinforcement in heel

$$c_{heel} = 35 \text{ mm}$$

Calculate shear for heel design

Shear from weight of base

$$V_{heel_wt_base} = \gamma_{f_d} \times \gamma_{base} \times l_{heel} \times t_{base} = 7.5 \text{ kN/m}$$

Shear from weight of moist backfill

$$V_{heel_wt_m} = w_{m_w_f} = 44.2 \text{ kN/m}$$

Shear from surcharge

$$V_{heel_sur} = w_{sur_f} = 2.6 \text{ kN/m}$$

Total shear for heel design

$$V_{heel} = V_{heel_wt_base} + V_{heel_wt_m} + V_{heel_sur} = 54.3 \text{ kN/m}$$

Calculate moment for heel design

Moment from weight of base

$$M_{heel_wt_base} = (\gamma_{f_d} \times \gamma_{base} \times t_{base} \times (l_{heel} + t_{wall} / 2)^2 / 2) = 3.9 \text{ kNm/m}$$

Moment from weight of moist backfill

$$M_{heel_wt_m} = w_{m_w_f} \times (l_{heel} + t_{wall}) / 2 = 22.1 \text{ kNm/m}$$

Moment from surcharge

$$M_{heel_sur} = w_{sur_f} \times (l_{heel} + t_{wall}) / 2 = 1.3 \text{ kNm/m}$$

Total moment for heel design

$$M_{heel} = M_{heel_wt_base} + M_{heel_wt_m} + M_{heel_sur} = 27.3 \text{ kNm/m}$$

200



Check heel in bending

Width of heel

$$b = 1000 \text{ mm/m}$$

Depth of reinforcement

$$d_{heel} = t_{base} - c_{heel} - (\phi_{heel} / 2) = 305.0 \text{ mm}$$

Constant

$$K_{heel} = M_{heel} / (b \times d_{heel}^2 \times f_{cu}) = 0.008$$

Compression reinforcement is not required

Lever arm

$$z_{heel} = \min(0.5 + \sqrt{(0.25 - (\min(K_{heel}, 0.225) / 0.9))}, 0.95) \times d_{heel}$$

$z_{heel} = 290 \text{ mm}$

Area of tension reinforcement required

$$A_{s_heel_des} = M_{heel} / (0.87 \times f_y \times z_{heel}) = 217 \text{ mm}^2/\text{m}$$

Minimum area of tension reinforcement

$$A_{s_heel_min} = k \times b \times t_{base} = 455 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required

$$A_{s_heel_req} = \text{Max}(A_{s_heel_des}, A_{s_heel_min}) = 455 \text{ mm}^2/\text{m}$$

Reinforcement provided

20 mm dia.bars @ 200 mm centres

Area of reinforcement provided

$$A_{s_heel_prov} = 1571 \text{ mm}^2/\text{m}$$

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PASS - Reinforcement provided at the retaining wall heel is adequate

Check shear resistance at heel

Design shear stress

$$v_{heel} = V_{heel} / (b \times d_{heel}) = 0.178 \text{ N/mm}^2$$

Allowable shear stress

$$v_{adm} = \min(0.8 \times \sqrt{f_{cu} / 1 \text{ N/mm}^2}, 5) \times 1 \text{ N/mm}^2 = 4.733 \text{ N/mm}^2$$

PASS - Design shear stress is less than maximum shear stress

From BS8110:Part 1:1997 – Table 3.8

Design concrete shear stress

$$v_{c_heel} = 0.606 \text{ N/mm}^2$$

$v_{heel} < v_{c_heel}$ - No shear reinforcement required

Design of reinforced concrete retaining wall stem (BS 8002:1994)

Material properties

Characteristic strength of concrete

$$f_{cu} = 35 \text{ N/mm}^2$$

Characteristic strength of reinforcement

$$f_y = 500 \text{ N/mm}^2$$

Wall details

Minimum area of reinforcement

$$k = 0.13 \%$$

Cover to reinforcement in stem

$$c_{stem} = 35 \text{ mm}$$

Cover to reinforcement in wall

$$c_{wall} = 35 \text{ mm}$$

Factored horizontal at-rest forces on stem

Surcharge

$$F_{s_sur_f} = \gamma_{f_l} \times K_0 \times \text{Surcharge} \times (h_{eff} - t_{base} - d_{ds}) = 6.4 \text{ kN/m}$$

Moist backfill above water table

$$F_{s_m_a_f} = 0.5 \times \gamma_{f_e} \times K_0 \times \gamma_m \times (h_{eff} - t_{base} - d_{ds} - h_{sat})^2 = 54.2 \text{ kN/m}$$

Calculate shear for stem design

Shear at base of stem

$$V_{stem} = F_{s_sur_f} + F_{s_m_a_f} = 60.6 \text{ kN/m}$$

Calculate moment for stem design

Surcharge

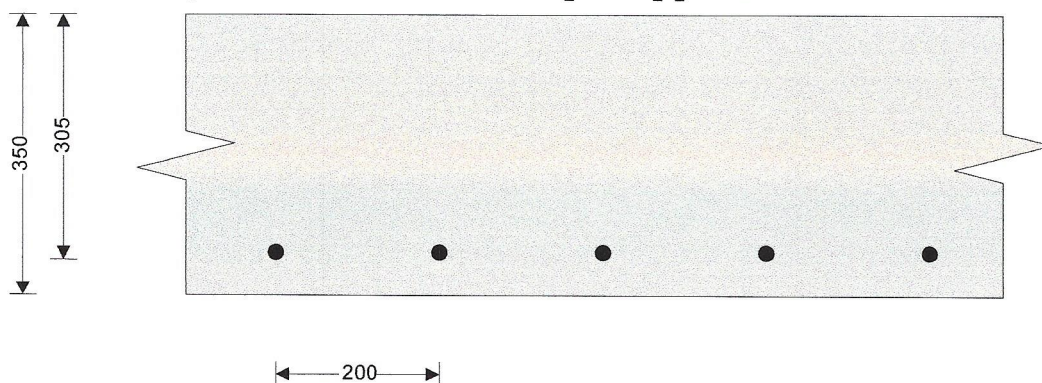
$$M_{s_sur} = F_{s_sur_f} \times (h_{stem} + t_{base}) / 2 = 9.7 \text{ kNm/m}$$

Moist backfill above water table

$$M_{s_m_a} = F_{s_m_a_f} \times (2 \times h_{sat} + h_{eff} - d_{ds} + t_{base} / 2) / 3 = 58.3 \text{ kNm/m}$$

Total moment for stem design

$$M_{stem} = M_{s_sur} + M_{s_m_a} = 68 \text{ kNm/m}$$



Check wall stem in bending

Width of wall stem

$$b = 1000 \text{ mm/m}$$

Depth of reinforcement

$$d_{stem} = t_{wall} - c_{stem} - (\phi_{stem} / 2) = 305.0 \text{ mm}$$

Constant

$$K_{stem} = M_{stem} / (b \times d_{stem}^2 \times f_{cu}) = 0.021$$

Compression reinforcement is not required

Lever arm

$$z_{stem} = \min(0.5 + \sqrt{(0.25 - (\min(K_{stem}, 0.225) / 0.9))}, 0.95) \times d_{stem}$$

$z_{stem} = 290 \text{ mm}$



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Area of tension reinforcement required

$$A_{s_stem_des} = M_{stem} / (0.87 \times f_y \times z_{stem}) = 539 \text{ mm}^2/\text{m}$$

Minimum area of tension reinforcement

$$A_{s_stem_min} = k \times b \times t_{wall} = 455 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required

$$A_{s_stem_req} = \text{Max}(A_{s_stem_des}, A_{s_stem_min}) = 539 \text{ mm}^2/\text{m}$$

Reinforcement provided

20 mm dia.bars @ 200 mm centres

Area of reinforcement provided

$$A_{s_stem_prov} = 1571 \text{ mm}^2/\text{m}$$

PASS - Reinforcement provided at the retaining wall stem is adequate

Check shear resistance at wall stem

Design shear stress

$$v_{stem} = V_{stem} / (b \times d_{stem}) = 0.199 \text{ N/mm}^2$$

Allowable shear stress

$$v_{adm} = \min(0.8 \times \sqrt{f_{cu}} / 1 \text{ N/mm}^2, 5) \times 1 \text{ N/mm}^2 = 4.733 \text{ N/mm}^2$$

PASS - Design shear stress is less than maximum shear stress

From BS8110:Part 1:1997 – Table 3.8

Design concrete shear stress

$$v_{c_stem} = 0.606 \text{ N/mm}^2$$

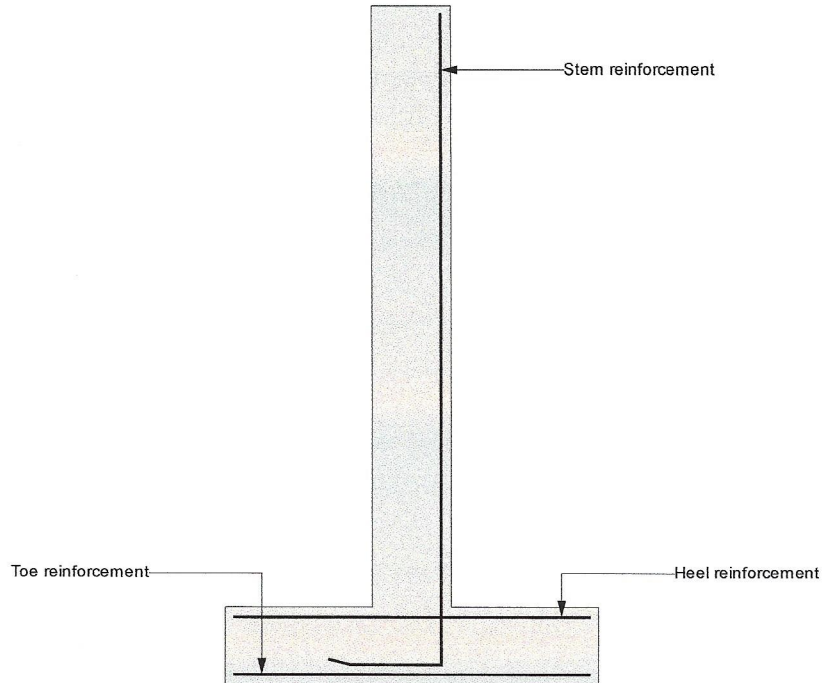
$v_{stem} < v_{c_stem}$ - No shear reinforcement required



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Indicative retaining wall reinforcement diagram



Toe bars - 20 mm dia.@ 200 mm centres - (1571 mm²/m)
Heel bars - 20 mm dia.@ 200 mm centres - (1571 mm²/m)
Stem bars - 20 mm dia.@ 200 mm centres - (1571 mm²/m)