



Cwm Planning & Design Ltd.
(Civil, Structural & Architectural)

STRUCTURAL CALCULATIONS

Masonry Retaining wall

at:

**69 Edward Street
Alltwen**

Date: 6th September 2022

BRIEF:

- 1) Produce design calculations for a masonry retaining wall at the front of the property at 68 Edward Street, Alltwen.

REFERENCES:

TEDDS structural computer package.

DESIGN LOADINGS:

Refer to structural calculations attached.

Surcharge = **2.5kN (Garden)**
Loads from exiting dwelling

SUMMARY

Retaining Wall – supporting path and front Garden)

Retained height taken as **1250mm** (Stem Height = 1450mm / 200mm cover to front)

**RETAINING WALL
DESIGN CALCULATIONS**

Project		69 Edward Street, Alltwen		Job no.	
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RETAINING WALL ANALYSIS

In accordance with EN1997-1:2004 incorporating Corrigendum dated February 2009 and the UK National Annex incorporating Corrigendum No.1

Tedds calculation version 2.9.14

Analysis summary

Description	Unit	Capacity	Applied	F o S	Result
Sliding stability	kN/m	26.5	14.8	1.789	PASS
Overturning stability	kNm/m	22	9.8	2.234	PASS
Bearing pressure	kN/m ²	150	59.7	2.514	PASS

Retaining wall details

Stem type	Cantilever with stepped rear face
Stem height	$h_{\text{stem}} = 1455 \text{ mm}$
Number of steps	$N_{\text{steps}} = 3$
Height of step 1	$h_{s1} = 225 \text{ mm}$
Thickness of step 1	$t_{s1} = 700 \text{ mm}$
Height of step 2	$h_{s2} = 900 \text{ mm}$
Thickness of step 2	$t_{s2} = 450 \text{ mm}$
Height of step 3	$h_{s3} = 330 \text{ mm}$
Thickness of step 3	$t_{s3} = 215 \text{ mm}$
Angle to rear face of stem	$\alpha = 90 \text{ deg}$
Stem density	$\gamma_{\text{stem}} = 25 \text{ kN/m}^3$
Toe length	$l_{\text{toe}} = 150 \text{ mm}$
Heel length	$l_{\text{heel}} = 150 \text{ mm}$
Base thickness	$t_{\text{base}} = 300 \text{ mm}$
Base density	$\gamma_{\text{base}} = 25 \text{ kN/m}^3$
Height of retained soil	$h_{\text{ret}} = 1255 \text{ mm}$
Angle of soil surface	$\beta = 0 \text{ deg}$
Depth of cover	$d_{\text{cover}} = 200 \text{ mm}$

Retained soil properties

Soil type	Medium dense well graded sand
Moist density	$\gamma_{\text{mr}} = 21 \text{ kN/m}^3$
Saturated density	$\gamma_{\text{sr}} = 23 \text{ kN/m}^3$
Characteristic effective shear resistance angle	$\phi'_{r,k} = 30 \text{ deg}$
Characteristic wall friction angle	$\delta_{r,k} = 0 \text{ deg}$

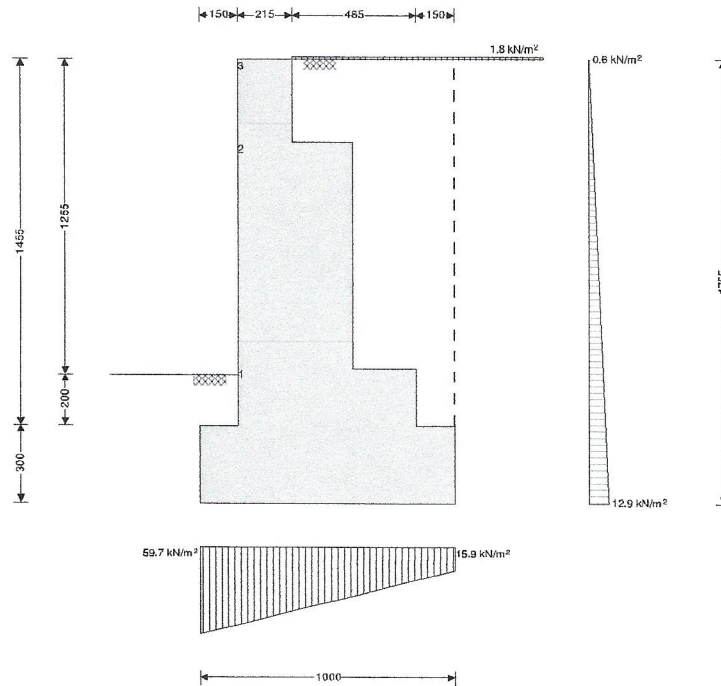
Base soil properties

Soil type	Medium dense well graded sand
Soil density	$\gamma_b = 22.5 \text{ kN/m}^3$
Characteristic effective shear resistance angle	$\phi'_{b,k} = 30 \text{ deg}$
Characteristic wall friction angle	$\delta_{b,k} = 15 \text{ deg}$
Characteristic base friction angle	$\delta_{bb,k} = 30 \text{ deg}$
Presumed bearing capacity	$P_{\text{bearing}} = 150 \text{ kN/m}^2$

Loading details

Permanent surcharge load	Surcharge _G = 0.3 kN/m ²
Variable surcharge load	Surcharge _Q = 1.5 kN/m ²

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General arrangement

Calculate retaining wall geometry

Base length

$$l_{base} = l_{toe} + t_{s1} + l_{heel} = 1000 \text{ mm}$$

Moist soil height

$$h_{moist} = h_{soil} = 1455 \text{ mm}$$

Length of surcharge load

$$l_{sur} = (l_{heel} + t_{s1} - t_{s3}) = 635 \text{ mm}$$

- Distance to vertical component

$$x_{sur_v} = l_{base} - (l_{heel} + t_{s1} - t_{s3}) / 2 = 682 \text{ mm}$$

Effective height of wall

$$h_{eff} = h_{base} + d_{cover} + h_{ret} = 1755 \text{ mm}$$

- Distance to horizontal component

$$x_{sur_h} = h_{eff} / 2 = 878 \text{ mm}$$

Area of wall stem

$$A_{stem} = h_{s1} \times t_{s1} + h_{s2} \times t_{s2} + h_{s3} \times t_{s3} = 0.633 \text{ m}^2$$

- Distance to vertical component

$$x_{stem} = l_{base} - (h_{s1} \times t_{s1} \times (l_{heel} + t_{s1} - t_{s1} / 2) + h_{s2} \times t_{s2} \times (l_{heel} + t_{s1} - t_{s2} / 2) + h_{s3} \times t_{s3} \times (l_{heel} + t_{s1} - t_{s3} / 2)) / A_{stem} = 393 \text{ mm}$$

Area of wall base

$$A_{base} = l_{base} \times t_{base} = 0.3 \text{ m}^2$$

- Distance to vertical component

$$x_{base} = l_{base} / 2 = 500 \text{ mm}$$

Area of moist soil

$$A_{moist} = l_{heel} \times h_{s1} + (l_{heel} + t_{s1} - t_{s2}) \times h_{s2} + (l_{heel} + t_{s1} - t_{s3}) \times h_{s3} = 0.603 \text{ m}^2$$

- Distance to vertical component

$$x_{moist_v} = l_{base} - (l_{heel} \times h_{s1} \times l_{heel} / 2 + (l_{heel} + t_{s1} - t_{s2}) \times h_{s2} \times (l_{heel} + t_{s1} - t_{s2} / 2) + (l_{heel} + t_{s1} - t_{s3}) \times h_{s3} \times (l_{heel} + t_{s1} - t_{s3} / 2)) / A_{moist} = 766 \text{ mm}$$

- Distance to horizontal component

$$x_{moist_h} = h_{eff} / 3 = 585 \text{ mm}$$

Area of base soil

$$A_{pass} = d_{cover} \times l_{toe} = 0.03 \text{ m}^2$$

- Distance to vertical component

$$x_{pass_v} = l_{base} - (d_{cover} \times l_{toe} \times (l_{base} - l_{toe} / 2)) / A_{pass} = 75 \text{ mm}$$

- Distance to horizontal component

$$x_{pass_h} = (d_{cover} + h_{base}) / 3 = 167 \text{ mm}$$

Area of excavated base soil

$$A_{exc} = h_{pass} \times l_{toe} = 0.03 \text{ m}^2$$

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- Distance to vertical component
- Distance to horizontal component

$$x_{exc_v} = l_{base} - (h_{pass} \times l_{toe} \times (l_{base} - l_{toe} / 2)) / A_{exc} = 75 \text{ mm}$$

$$x_{exc_h} = (h_{pass} + h_{base}) / 3 = 167 \text{ mm}$$

Using Coulomb theory

Active pressure coefficient

$$K_A = \sin(\alpha + \phi'_{r,k})^2 / (\sin(\alpha)^2 \times \sin(\alpha - \delta_{r,k}) \times [1 + \sqrt{[\sin(\phi'_{r,k} + \delta_{r,k}) \times \sin(\phi'_{r,k} - \beta) / (\sin(\alpha - \delta_{r,k}) \times \sin(\alpha + \beta))]}]^2) = 0.333$$

Passive pressure coefficient

$$K_P = \sin(90 - \phi'_{b,k})^2 / (\sin(90 + \delta_{b,k}) \times [1 - \sqrt{[\sin(\phi'_{b,k} + \delta_{b,k}) \times \sin(\phi'_{b,k}) / (\sin(90 + \delta_{b,k}))]}]^2) = 4.977$$

Bearing pressure check

Vertical forces on wall

Wall stem

$$F_{stem} = A_{stem} \times \gamma_{stem} = 15.8 \text{ kN/m}$$

Wall base

$$F_{base} = A_{base} \times \gamma_{base} = 7.5 \text{ kN/m}$$

Surcharge load

$$F_{sur_v} = (Surcharge_G + Surcharge_Q) \times (l_{heel} + t_{s1} - t_{s3}) = 1.1 \text{ kN/m}$$

Moist retained soil

$$F_{moist_v} = A_{moist} \times \gamma_{mr} = 12.7 \text{ kN/m}$$

Base soil

$$F_{pass_v} = A_{pass} \times \gamma_b = 0.7 \text{ kN/m}$$

Total

$$F_{total_v} = F_{stem} + F_{base} + F_{sur_v} + F_{moist_v} + F_{pass_v} = 37.8 \text{ kN/m}$$

Horizontal forces on wall

Surcharge load

$$F_{sur_h} = K_A \times (Surcharge_G + Surcharge_Q) \times h_{eff} = 1 \text{ kN/m}$$

Moist retained soil

$$F_{moist_h} = K_A \times \gamma_{mr} \times h_{eff}^2 / 2 = 10.8 \text{ kN/m}$$

Base soil

$$F_{pass_h} = \max(-K_P \times \cos(\delta_{b,k}) \times \gamma_b \times (d_{cover} + h_{base})^2 / 2, -(F_{moist_h} + F_{sur_h})) = -11.8 \text{ kN/m}$$

Total

$$F_{total_h} = \max(F_{sur_h} + F_{moist_h} + F_{pass_h} - F_{total_v} \times \tan(\delta_{bb,k}), 0 \text{ kN/m}) = 0 \text{ kN/m}$$

Moments on wall

Wall stem

$$M_{stem} = F_{stem} \times x_{stem} = 6.2 \text{ kNm/m}$$

Wall base

$$M_{base} = F_{base} \times x_{base} = 3.8 \text{ kNm/m}$$

Surcharge load

$$M_{sur} = F_{sur_v} \times x_{sur_v} - F_{sur_h} \times x_{sur_h} = -0.1 \text{ kNm/m}$$

Moist retained soil

$$M_{moist} = F_{moist_v} \times x_{moist_v} - F_{moist_h} \times x_{moist_h} = 3.4 \text{ kNm/m}$$

Base soil

$$M_{pass} = F_{pass_v} \times x_{pass_v} - F_{pass_h} \times x_{pass_h} = 2 \text{ kNm/m}$$

Total

$$M_{total} = M_{stem} + M_{base} + M_{sur} + M_{moist} + M_{pass} = 15.3 \text{ kNm/m}$$

Check bearing pressure

Distance to reaction

$$\bar{x} = M_{total} / F_{total_v} = 404 \text{ mm}$$

Eccentricity of reaction

$$e = \bar{x} - l_{base} / 2 = -96 \text{ mm}$$

Loaded length of base

$$l_{load} = l_{base} = 1000 \text{ mm}$$

Bearing pressure at toe

$$q_{toe} = F_{total_v} / l_{base} \times (1 - 6 \times e / l_{base}) = 59.7 \text{ kN/m}^2$$

Bearing pressure at heel

$$q_{heel} = F_{total_v} / l_{base} \times (1 + 6 \times e / l_{base}) = 15.9 \text{ kN/m}^2$$

Factor of safety

$$FoS_{bp} = P_{bearing} / \max(q_{toe}, q_{heel}) = 2.514$$

PASS - Allowable bearing pressure exceeds maximum applied bearing pressure

Design approach 1

Partial factors on actions - Table A.3 - Combination 1

Partial factor set

$$A1$$

Permanent unfavourable action

$$\gamma_G = 1.35$$

Permanent favourable action

$$\gamma_{Gf} = 1.00$$

Variable unfavourable action

$$\gamma_Q = 1.50$$

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Variable favourable action

$$\gamma_{Qr} = 0.00$$

Partial factors for soil parameters – Table A.4 - Combination 1

Soil parameter set	M1
Angle of shearing resistance	$\gamma_{\phi'} = 1.00$
Effective cohesion	$\gamma_{c'} = 1.00$
Weight density	$\gamma_r = 1.00$

Library item Partial factors output

Retained soil properties

Design moist density	$\gamma_{mr}' = \gamma_{mr} / \gamma_r = 21 \text{ kN/m}^3$
Design saturated density	$\gamma_{sr}' = \gamma_{sr} / \gamma_r = 23 \text{ kN/m}^3$
Design effective shear resistance angle	$\phi'_{r,d} = \text{atan}(\tan(\phi'_{r,k}) / \gamma_{\phi'}) = 30 \text{ deg}$
Design wall friction angle	$\delta_{r,d} = \text{atan}(\tan(\delta_{r,k}) / \gamma_{\phi'}) = 0 \text{ deg}$

Base soil properties

Design soil density	$\gamma_b' = \gamma_b / \gamma_r = 22.5 \text{ kN/m}^3$
Design effective shear resistance angle	$\phi'_{b,d} = \text{atan}(\tan(\phi'_{b,k}) / \gamma_{\phi'}) = 30 \text{ deg}$
Design wall friction angle	$\delta_{b,d} = \text{atan}(\tan(\delta_{b,k}) / \gamma_{\phi'}) = 15 \text{ deg}$
Design base friction angle	$\delta_{bb,d} = \text{atan}(\tan(\delta_{bb,k}) / \gamma_{\phi'}) = 30 \text{ deg}$
Design effective cohesion	$c'_{b,d} = c'_{b,k} / \gamma_{c'} = 0 \text{ kN/m}^2$

Using Coulomb theory

Active pressure coefficient	$K_A = \sin(\alpha + \phi'_{r,d})^2 / (\sin(\alpha)^2 \times \sin(\alpha - \delta_{r,d}) \times [1 + \sqrt{\sin(\phi'_{r,d} + \delta_{r,d}) \times \sin(\phi'_{r,d} - \beta)} / (\sin(\alpha - \delta_{r,d}) \times \sin(\alpha + \beta))])^2 = 0.333$
Passive pressure coefficient	$K_P = \sin(90 - \phi'_{b,d})^2 / (\sin(90 + \delta_{b,d}) \times [1 - \sqrt{\sin(\phi'_{b,d} + \delta_{b,d}) \times \sin(\phi'_{b,d} - \beta)} / (\sin(90 + \delta_{b,d}))])^2 = 4.977$

Sliding check

Vertical forces on wall

Wall stem	$F_{stem} = \gamma_{Gf} \times A_{stem} \times \gamma_{stem} = 15.8 \text{ kN/m}$
Wall base	$F_{base} = \gamma_{Gf} \times A_{base} \times \gamma_{base} = 7.5 \text{ kN/m}$
Moist retained soil	$F_{moist_v} = \gamma_{Gf} \times A_{moist} \times \gamma_{mr}' = 12.7 \text{ kN/m}$
Base soil	$F_{exc_v} = \gamma_{Gf} \times A_{exc} \times \gamma_b' = 0.7 \text{ kN/m}$
Total	$F_{total_v} = F_{stem} + F_{base} + F_{moist_v} + F_{exc_v} = 36.7 \text{ kN/m}$

Horizontal forces on wall

Surcharge load	$F_{sur_h} = K_A \times (\gamma_G \times \text{Surcharge}_G + \gamma_Q \times \text{Surcharge}_Q) \times h_{eff} = 1.5 \text{ kN/m}$
Moist retained soil	$F_{moist_h} = \gamma_G \times K_A \times \gamma_{mr}' \times h_{eff}^2 / 2 = 14.6 \text{ kN/m}$
Total	$F_{total_h} = F_{sur_h} + F_{moist_h} = 16.1 \text{ kN/m}$

Check stability against sliding

Base soil resistance	$F_{exc_h} = \gamma_{Gf} \times K_P \times \cos(\delta_{b,d}) \times \gamma_b' \times (h_{pass} + h_{base})^2 / 2 = 13.5 \text{ kN/m}$
Base friction	$F_{friction} = F_{total_v} \times \tan(\delta_{bb,d}) = 21.2 \text{ kN/m}$
Resistance to sliding	$F_{rest} = F_{exc_h} + F_{friction} = 34.7 \text{ kN/m}$
Factor of safety	$FoS_{sl} = F_{rest} / F_{total_h} = 2.16$

PASS - Resistance to sliding is greater than sliding force

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Overturning check

Vertical forces on wall

Wall stem	$F_{stem} = \gamma_{Gf} \times A_{stem} \times \gamma_{stem} = 15.8 \text{ kN/m}$
Wall base	$F_{base} = \gamma_{Gf} \times A_{base} \times \gamma_{base} = 7.5 \text{ kN/m}$
Moist retained soil	$F_{moist_v} = \gamma_{Gf} \times A_{moist} \times \gamma_{mr}' = 12.7 \text{ kN/m}$
Base soil	$F_{exc_v} = \gamma_{Gf} \times A_{exc} \times \gamma_b' = 0.7 \text{ kN/m}$
Total	$F_{total_v} = F_{stem} + F_{base} + F_{moist_v} + F_{exc_v} = 36.7 \text{ kN/m}$

Horizontal forces on wall

Surcharge load	$F_{sur_h} = K_A \times (\gamma_G \times \text{Surcharge}_G + \gamma_Q \times \text{Surcharge}_Q) \times h_{eff} = 1.5 \text{ kN/m}$
Moist retained soil	$F_{moist_h} = \gamma_G \times K_A \times \gamma_{mr}' \times h_{eff}^2 / 2 = 14.6 \text{ kN/m}$
Base soil	$F_{exc_h} = -\gamma_{Gf} \times K_P \times \cos(\delta_{b,d}) \times \gamma_b' \times (h_{pass} + h_{base})^2 / 2 = -13.5 \text{ kN/m}$
Total	$F_{total_h} = F_{sur_h} + F_{moist_h} + F_{exc_h} = 2.5 \text{ kN/m}$

Overturning moments on wall

Surcharge load	$M_{sur_OT} = F_{sur_h} \times x_{sur_h} = 1.3 \text{ kNm/m}$
Moist retained soil	$M_{moist_OT} = F_{moist_h} \times x_{moist_h} = 8.5 \text{ kNm/m}$
Total	$M_{total_OT} = M_{sur_OT} + M_{moist_OT} = 9.8 \text{ kNm/m}$

Restoring moments on wall

Wall stem	$M_{stem_R} = F_{stem} \times x_{stem} = 6.2 \text{ kNm/m}$
Wall base	$M_{base_R} = F_{base} \times x_{base} = 3.8 \text{ kNm/m}$
Moist retained soil	$M_{moist_R} = F_{moist_v} \times x_{moist_v} = 9.7 \text{ kNm/m}$
Base soil	$M_{exc_R} = F_{exc_v} \times x_{exc_v} - F_{exc_h} \times x_{exc_h} = 2.3 \text{ kNm/m}$
Total	$M_{total_R} = M_{stem_R} + M_{base_R} + M_{moist_R} + M_{exc_R} = 22 \text{ kNm/m}$

Check stability against overturning

Factor of safety	$FoS_{OT} = M_{total_R} / M_{total_OT} = 2.234$
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PASS - Maximum restoring moment is greater than overturning moment

Design approach 1

Partial factors on actions - Table A.3 - Combination 2

Partial factor set	A2
Permanent unfavourable action	$\gamma_G = 1.00$
Permanent favourable action	$\gamma_{Gf} = 1.00$
Variable unfavourable action	$\gamma_Q = 1.30$
Variable favourable action	$\gamma_{Qf} = 0.00$

Partial factors for soil parameters – Table A.4 - Combination 2

Soil parameter set	M2
Angle of shearing resistance	$\gamma_{\phi'} = 1.25$
Effective cohesion	$\gamma_c = 1.25$
Weight density	$\gamma_\gamma = 1.00$

Library item Partial factors output

Retained soil properties

Design moist density	$\gamma_{mr}' = \gamma_{mr} / \gamma_\gamma = 21 \text{ kN/m}^3$
Design saturated density	$\gamma_{sr}' = \gamma_{sr} / \gamma_\gamma = 23 \text{ kN/m}^3$
Design effective shear resistance angle	$\phi'_{r,d} = \text{atan}(\tan(\phi'_{r,k}) / \gamma_{\phi'}) = 24.8 \text{ deg}$

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Design wall friction angle

$$\delta_{r,d} = \text{atan}(\tan(\delta_{r,k}) / \gamma_{\phi}) = 0 \text{ deg}$$

Base soil properties

Design soil density

$$\gamma_b' = \gamma_b / \gamma_r = 22.5 \text{ kN/m}^3$$

Design effective shear resistance angle

$$\phi'_{b,d} = \text{atan}(\tan(\phi'_{b,k}) / \gamma_{\phi}) = 24.8 \text{ deg}$$

Design wall friction angle

$$\delta_{b,d} = \text{atan}(\tan(\delta_{b,k}) / \gamma_{\phi}) = 12.1 \text{ deg}$$

Design base friction angle

$$\delta_{bb,d} = \text{atan}(\tan(\delta_{bb,k}) / \gamma_{\phi}) = 24.8 \text{ deg}$$

Design effective cohesion

$$c'_{b,d} = c'_{b,k} / \gamma_c = 0 \text{ kN/m}^2$$

Using Coulomb theory

Active pressure coefficient

$$K_A = \sin(\alpha + \phi'_{r,d})^2 / (\sin(\alpha)^2 \times \sin(\alpha - \delta_{r,d}) \times [1 + \sqrt{\sin(\phi'_{r,d} + \delta_{r,d}) \times \sin(\phi'_{r,d} - \beta) / (\sin(\alpha - \delta_{r,d}) \times \sin(\alpha + \beta))}]^2) = 0.409$$

Passive pressure coefficient

$$K_P = \sin(90 - \phi'_{b,d})^2 / (\sin(90 + \delta_{b,d}) \times [1 - \sqrt{\sin(\phi'_{b,d} + \delta_{b,d}) \times \sin(\phi'_{b,d} - \beta) / (\sin(90 + \delta_{b,d}))}]^2) = 3.473$$

Sliding check

Vertical forces on wall

Wall stem

$$F_{\text{stem}} = \gamma_{Gf} \times A_{\text{stem}} \times \gamma_{\text{stem}} = 15.8 \text{ kN/m}$$

Wall base

$$F_{\text{base}} = \gamma_{Gf} \times A_{\text{base}} \times \gamma_{\text{base}} = 7.5 \text{ kN/m}$$

Moist retained soil

$$F_{\text{moist}_v} = \gamma_{Gf} \times A_{\text{moist}} \times \gamma_{mr}' = 12.7 \text{ kN/m}$$

Base soil

$$F_{\text{exc}_v} = \gamma_{Gf} \times A_{\text{exc}} \times \gamma_b' = 0.7 \text{ kN/m}$$

Total

$$F_{\text{total}_v} = F_{\text{stem}} + F_{\text{base}} + F_{\text{moist}_v} + F_{\text{exc}_v} = 36.7 \text{ kN/m}$$

Horizontal forces on wall

Surcharge load

$$F_{\text{sur}_h} = K_A \times (\gamma_G \times \text{Surcharge}_G + \gamma_Q \times \text{Surcharge}_Q) \times h_{\text{eff}} = 1.6 \text{ kN/m}$$

Moist retained soil

$$F_{\text{moist}_h} = \gamma_G \times K_A \times \gamma_{mr}' \times h_{\text{eff}}^2 / 2 = 13.2 \text{ kN/m}$$

Total

$$F_{\text{total}_h} = F_{\text{sur}_h} + F_{\text{moist}_h} = 14.8 \text{ kN/m}$$

Check stability against sliding

Base soil resistance

$$F_{\text{exc}_h} = \gamma_{Gf} \times K_P \times \cos(\delta_{b,d}) \times \gamma_b' \times (h_{\text{pass}} + h_{\text{base}})^2 / 2 = 9.6 \text{ kN/m}$$

Base friction

$$F_{\text{friction}} = F_{\text{total}_v} \times \tan(\delta_{bb,d}) = 16.9 \text{ kN/m}$$

Resistance to sliding

$$F_{\text{rest}} = F_{\text{exc}_h} + F_{\text{friction}} = 26.5 \text{ kN/m}$$

Factor of safety

$$FoS_{\text{Sl}} = F_{\text{rest}} / F_{\text{total}_h} = 1.789$$

PASS - Resistance to sliding is greater than sliding force

Overturning check

Vertical forces on wall

Wall stem

$$F_{\text{stem}} = \gamma_{Gf} \times A_{\text{stem}} \times \gamma_{\text{stem}} = 15.8 \text{ kN/m}$$

Wall base

$$F_{\text{base}} = \gamma_{Gf} \times A_{\text{base}} \times \gamma_{\text{base}} = 7.5 \text{ kN/m}$$

Moist retained soil

$$F_{\text{moist}_v} = \gamma_{Gf} \times A_{\text{moist}} \times \gamma_{mr}' = 12.7 \text{ kN/m}$$

Base soil

$$F_{\text{exc}_v} = \gamma_{Gf} \times A_{\text{exc}} \times \gamma_b' = 0.7 \text{ kN/m}$$

Total

$$F_{\text{total}_v} = F_{\text{stem}} + F_{\text{base}} + F_{\text{moist}_v} + F_{\text{exc}_v} = 36.7 \text{ kN/m}$$

Horizontal forces on wall

Surcharge load

$$F_{\text{sur}_h} = K_A \times (\gamma_G \times \text{Surcharge}_G + \gamma_Q \times \text{Surcharge}_Q) \times h_{\text{eff}} = 1.6 \text{ kN/m}$$

Moist retained soil

$$F_{\text{moist}_h} = \gamma_G \times K_A \times \gamma_{mr}' \times h_{\text{eff}}^2 / 2 = 13.2 \text{ kN/m}$$

Base soil

$$F_{\text{exc}_h} = -\gamma_{Gf} \times K_P \times \cos(\delta_{b,d}) \times \gamma_b' \times (h_{\text{pass}} + h_{\text{base}})^2 / 2 = -9.6 \text{ kN/m}$$

Total

$$F_{\text{total}_h} = F_{\text{sur}_h} + F_{\text{moist}_h} + F_{\text{exc}_h} = 5.3 \text{ kN/m}$$

Project		69 Edward Street, Alltwen		Job no.	
Calcs for		GARDEN RETAINING WALL		Start page no./Revision	
				7	
Calcs by	Calcs date	Checked by	Checked date	Approved by	Approved date
CM	06/09/2022				

Overturning moments on wall

Surcharge load

$$M_{sur_OT} = F_{sur_h} \times x_{sur_h} = 1.4 \text{ kNm/m}$$

Moist retained soil

$$M_{moist_OT} = F_{moist_h} \times x_{moist_h} = 7.7 \text{ kNm/m}$$

Total

$$M_{total_OT} = M_{sur_OT} + M_{moist_OT} = 9.1 \text{ kNm/m}$$

Restoring moments on wall

Wall stem

$$M_{stem_R} = F_{stem} \times x_{stem} = 6.2 \text{ kNm/m}$$

Wall base

$$M_{base_R} = F_{base} \times x_{base} = 3.8 \text{ kNm/m}$$

Moist retained soil

$$M_{moist_R} = F_{moist_v} \times x_{moist_v} = 9.7 \text{ kNm/m}$$

Base soil

$$M_{exc_R} = F_{exc_v} \times x_{exc_v} - F_{exc_h} \times x_{exc_h} = 1.6 \text{ kNm/m}$$

Total

$$M_{total_R} = M_{stem_R} + M_{base_R} + M_{moist_R} + M_{exc_R} = 21.3 \text{ kNm/m}$$

Check stability against overturning

Factor of safety

$$FoS_{ot} = M_{total_R} / M_{total_OT} = 2.336$$

PASS - Maximum restoring moment is greater than overturning moment