

27 Swan Road, Baglan

Retaining Wall - Structural Calculations

02/07/2020

Solutions to Structures Ltd.

J.M. Davies

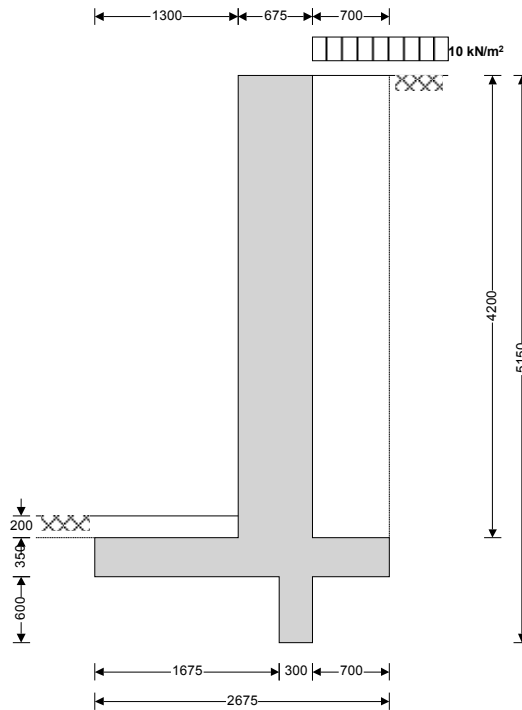
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Section Retaining Works				Sheet no./rev. 2	
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INTRODUCTION

Solutions to Structures Ltd. have been appointed to carry out the structural design for the proposed retaining works to the side of the above property shown on drawing series 20-062.

RETAINING WALL ANALYSIS (BS 8002:1994)

TEDDS calculation version 1.2.01.07



Wall details

Retaining wall type;
Height of retaining wall stem;
Thickness of wall stem;
Length of toe;
Length of heel;
Overall length of base;
Thickness of base;
Depth of downstand;
Position of downstand;
Thickness of downstand;
Height of retaining wall;
Depth of cover in front of wall;
Depth of unplanned excavation;
Height of ground water behind wall;
Height of saturated fill above base;
Density of wall construction;
Density of base construction;
Angle of rear face of wall;
Angle of soil surface behind wall;
Effective height at virtual back of wall;

Unpropped cantilever

$h_{\text{stem}} = 4200$ mm
 $t_{\text{wall}} = 675$ mm
 $l_{\text{toe}} = 1300$ mm
 $l_{\text{heel}} = 700$ mm
 $l_{\text{base}} = l_{\text{toe}} + l_{\text{heel}} + t_{\text{wall}} = 2675$ mm
 $t_{\text{base}} = 350$ mm
 $d_{\text{ds}} = 600$ mm
 $l_{\text{ds}} = 1675$ mm
 $t_{\text{ds}} = 300$ mm
 $h_{\text{wall}} = h_{\text{stem}} + t_{\text{base}} + d_{\text{ds}} = 5150$ mm
 $d_{\text{cover}} = 200$ mm
 $d_{\text{exc}} = 200$ mm
 $h_{\text{water}} = 0$ mm
 $h_{\text{sat}} = \max(h_{\text{water}} - t_{\text{base}} - d_{\text{ds}}, 0 \text{ mm}) = 0$ mm
 $\gamma_{\text{wall}} = 21.0$ kN/m³
 $\gamma_{\text{base}} = 24.0$ kN/m³
 $\alpha = 90.0$ deg
 $\beta = 0.0$ deg
 $h_{\text{eff}} = h_{\text{wall}} + l_{\text{heel}} \times \tan(\beta) = 5150$ mm

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Retained material details

Mobilisation factor;	$M = 1.5$
Moist density of retained material;	$\gamma_m = 18.0 \text{ kN/m}^3$
Saturated density of retained material;	$\gamma_s = 21.0 \text{ kN/m}^3$
Design shear strength;	$\phi' = 26.7 \text{ deg}$
Angle of wall friction;	$\delta = 20.7 \text{ deg}$

Base material details

Moist density;	$\gamma_{mb} = 18.0 \text{ kN/m}^3$
Design shear strength;	$\phi'_b = 26.7 \text{ deg}$
Design base friction;	$\delta_b = 20.7 \text{ deg}$
Allowable bearing pressure;	$P_{\text{bearing}} = 100 \text{ kN/m}^2$

Using Coulomb theory

Active pressure coefficient for retained material

$$K_a = \sin(\alpha + \phi')^2 / (\sin(\alpha)^2 \times \sin(\alpha - \delta) \times [1 + \sqrt{(\sin(\phi' + \delta) \times \sin(\phi' - \beta) / (\sin(\alpha - \delta) \times \sin(\alpha + \beta)))}]^2) = 0.336$$

Passive pressure coefficient for base material

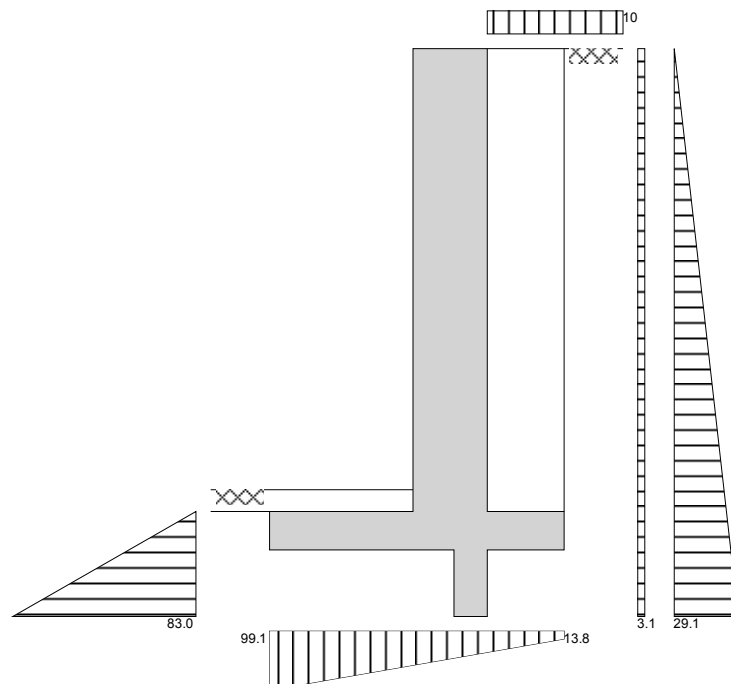
$$K_p = \sin(90 - \phi'_b)^2 / (\sin(90 - \delta_b) \times [1 - \sqrt{(\sin(\phi'_b + \delta_b) \times \sin(\phi'_b) / (\sin(90 + \delta_b)))}]^2) = 5.192$$

At-rest pressure

At-rest pressure for retained material;	$K_0 = 1 - \sin(\phi') = 0.551$
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Loading details

Surcharge load on plan;	Surcharge = 10.0 kN/m^2
Applied vertical dead load on wall;	$W_{\text{dead}} = 0.0 \text{ kN/m}$
Applied vertical live load on wall;	$W_{\text{live}} = 0.0 \text{ kN/m}$
Position of applied vertical load on wall;	$l_{\text{load}} = 0 \text{ mm}$
Applied horizontal dead load on wall;	$F_{\text{dead}} = 0.0 \text{ kN/m}$
Applied horizontal live load on wall;	$F_{\text{live}} = 0.0 \text{ kN/m}$
Height of applied horizontal load on wall;	$h_{\text{load}} = 0 \text{ mm}$



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Vertical forces on wall

Wall stem;	$w_{wall} = h_{stem} \times t_{wall} \times \gamma_{wall} = 59.5 \text{ kN/m}$
Wall base;	$w_{base} = l_{base} \times t_{base} \times \gamma_{base} = 22.5 \text{ kN/m}$
Wall downstand;	$w_{ds} = d_{ds} \times t_{ds} \times \gamma_{base} = 4.3 \text{ kN/m}$
Surcharge;	$w_{sur} = \text{Surcharge} \times l_{heel} = 7 \text{ kN/m}$
Moist backfill to top of wall;	$w_{m_w} = l_{heel} \times (h_{stem} - h_{sat}) \times \gamma_m = 52.9 \text{ kN/m}$
Soil in front of wall;	$w_p = l_{toe} \times d_{cover} \times \gamma_{mb} = 4.7 \text{ kN/m}$
Total vertical load;	$W_{total} = w_{wall} + w_{base} + w_{ds} + w_{sur} + w_{m_w} + w_p = 150.9 \text{ kN/m}$

Horizontal forces on wall

Surcharge;	$F_{sur} = K_a \times \cos(90 - \alpha + \delta) \times \text{Surcharge} \times h_{eff} = 16.2 \text{ kN/m}$
Moist backfill above water table;	$F_{m_a} = 0.5 \times K_a \times \cos(90 - \alpha + \delta) \times \gamma_m \times (h_{eff} - h_{water})^2 = 74.9 \text{ kN/m}$
Total horizontal load;	$F_{total} = F_{sur} + F_{m_a} = 91.1 \text{ kN/m}$

Calculate stability against sliding

Passive resistance of soil in front of wall;	$F_p = 0.5 \times K_p \times \cos(\delta_b) \times (d_{cover} + t_{base} + d_{ds} - d_{exc})^2 \times \gamma_{mb} = 39.4 \text{ kN/m}$
Resistance to sliding;	$F_{res} = F_p + (W_{total} - w_{sur} - w_p) \times \tan(\delta_b) = 92.1 \text{ kN/m}$
PASS - Resistance force is greater than sliding force	

Overturning moments

Surcharge;	$M_{sur} = F_{sur} \times (h_{eff} - 2 \times d_{ds}) / 2 = 31.9 \text{ kNm/m}$
Moist backfill above water table;	$M_{m_a} = F_{m_a} \times (h_{eff} + 2 \times h_{water} - 3 \times d_{ds}) / 3 = 83.7 \text{ kNm/m}$
Soil in front of wall;	$M_{p_o} = F_p \times [2 \times d_{ds} - t_{base} - d_{cover} + d_{exc}] / 3 = 11.2 \text{ kNm/m}$
Total overturning moment;	$M_{ot} = M_{sur} + M_{m_a} + M_{p_o} = 126.8 \text{ kNm/m}$

Restoring moments

Wall stem;	$M_{wall} = w_{wall} \times (l_{toe} + t_{wall} / 2) = 97.5 \text{ kNm/m}$
Wall base;	$M_{base} = w_{base} \times l_{base} / 2 = 30.1 \text{ kNm/m}$
Wall downstand;	$M_{ds} = w_{ds} \times (l_{ds} + t_{ds} / 2) = 7.9 \text{ kNm/m}$
Moist backfill;	$M_{m_r} = (w_{m_w} \times (l_{base} - l_{heel} / 2) + w_{m_s} \times (l_{base} - l_{heel} / 3)) = 123 \text{ kNm/m}$
Total restoring moment;	$M_{rest} = M_{wall} + M_{base} + M_{ds} + M_{m_r} = 258.5 \text{ kNm/m}$

Check stability against overturning

Total overturning moment;	$M_{ot} = 126.8 \text{ kNm/m}$
Total restoring moment;	$M_{rest} = 258.5 \text{ kNm/m}$
PASS - Restoring moment is greater than overturning moment	

Check bearing pressure

Surcharge;	$M_{sur_r} = w_{sur} \times (l_{base} - l_{heel} / 2) = 16.3 \text{ kNm/m}$
Soil in front of wall;	$M_{p_r} = w_p \times l_{toe} / 2 = 3 \text{ kNm/m}$
Total moment for bearing;	$M_{total} = M_{rest} - M_{ot} + M_{sur_r} + M_{p_r} = 151 \text{ kNm/m}$
Total vertical reaction;	$R = W_{total} = 150.9 \text{ kN/m}$
Distance to reaction;	$x_{bar} = M_{total} / R = 1001 \text{ mm}$
Eccentricity of reaction;	$e = \text{abs}((l_{base} / 2) - x_{bar}) = 337 \text{ mm}$
Reaction acts within middle third of base	
Bearing pressure at toe;	$p_{toe} = (R / l_{base}) + (6 \times R \times e / l_{base}^2) = 99.1 \text{ kN/m}^2$
Bearing pressure at heel;	$p_{heel} = (R / l_{base}) - (6 \times R \times e / l_{base}^2) = 13.8 \text{ kN/m}^2$
PASS - Maximum bearing pressure is less than allowable bearing pressure	

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RETAINING WALL DESIGN (BS 8002:1994)

TEDDS calculation version 1.2.01.07

Ultimate limit state load factors

Dead load factor; $\gamma_{f,d} = 1.4$
Live load factor; $\gamma_{f,l} = 1.6$
Earth and water pressure factor; $\gamma_{f,e} = 1.4$

Factored vertical forces on wall

Wall stem; $W_{wall,f} = \gamma_{f,d} \times h_{stem} \times t_{wall} \times \gamma_{wall} = 83.3 \text{ kN/m}$
Wall base; $W_{base,f} = \gamma_{f,d} \times l_{base} \times t_{base} \times \gamma_{base} = 31.5 \text{ kN/m}$
Wall downstand; $W_{ds,f} = \gamma_{f,d} \times d_{ds} \times t_{ds} \times \gamma_{base} = 6 \text{ kN/m}$
Surcharge; $W_{sur,f} = \gamma_{f,l} \times \text{Surcharge} \times l_{heel} = 11.2 \text{ kN/m}$
Moist backfill to top of wall; $W_{m,w,f} = \gamma_{f,d} \times l_{heel} \times (h_{stem} - h_{sat}) \times \gamma_m = 74.1 \text{ kN/m}$
Soil in front of wall; $W_{p,f} = \gamma_{f,d} \times l_{toe} \times d_{cover} \times \gamma_{mb} = 6.6 \text{ kN/m}$
Total vertical load; $W_{total,f} = W_{wall,f} + W_{base,f} + W_{ds,f} + W_{sur,f} + W_{m,w,f} + W_{p,f} = 212.7 \text{ kN/m}$

Factored horizontal at-rest forces on wall

Surcharge; $F_{sur,f} = \gamma_{f,l} \times K_0 \times \text{Surcharge} \times h_{eff} = 45.4 \text{ kN/m}$
Moist backfill above water table; $F_{m,a,f} = \gamma_{f,e} \times 0.5 \times K_0 \times \gamma_m \times (h_{eff} - h_{water})^2 = 184 \text{ kN/m}$
Total horizontal load; $F_{total,f} = F_{sur,f} + F_{m,a,f} = 229.4 \text{ kN/m}$
Passive resistance of soil in front of wall; $F_{p,f} = \gamma_{f,e} \times 0.5 \times K_p \times \cos(\delta_b) \times (d_{cover} + t_{base} + d_{ds} - d_{exc})^2 \times \gamma_{mb} = 55.2 \text{ kN/m}$

Factored overturning moments

Surcharge; $M_{sur,f} = F_{sur,f} \times (h_{eff} - 2 \times d_{ds}) / 2 = 89.6 \text{ kNm/m}$
Moist backfill above water table; $M_{m,a,f} = F_{m,a,f} \times (h_{eff} + 2 \times h_{water} - 3 \times d_{ds}) / 3 = 205.5 \text{ kNm/m}$
Soil in front of wall; $M_{p,o,f} = F_{p,f} \times [2 \times d_{ds} - t_{base} - d_{cover} + d_{exc}] / 3 = 15.6 \text{ kNm/m}$
Total overturning moment; $M_{ot,f} = M_{sur,f} + M_{m,a,f} + M_{p,o,f} = 310.8 \text{ kNm/m}$

Restoring moments

Wall stem; $M_{wall,f} = W_{wall,f} \times (l_{toe} + t_{wall} / 2) = 136.5 \text{ kNm/m}$
Wall base; $M_{base,f} = W_{base,f} \times l_{base} / 2 = 42.1 \text{ kNm/m}$
Wall downstand; $M_{ds,f} = W_{ds,f} \times (l_{ds} + t_{ds} / 2) = 11 \text{ kNm/m}$
Surcharge; $M_{sur,r,f} = W_{sur,f} \times (l_{base} - l_{heel} / 2) = 26 \text{ kNm/m}$
Moist backfill; $M_{m,r,f} = (W_{m,w,f} \times (l_{base} - l_{heel} / 2) + W_{m,s,f} \times (l_{base} - l_{heel} / 3)) = 172.3 \text{ kNm/m}$
Soil in front of wall; $M_{p,r,f} = W_{p,f} \times l_{toe} / 2 = 4.3 \text{ kNm/m}$
Total restoring moment; $M_{rest,f} = M_{wall,f} + M_{base,f} + M_{ds,f} + M_{sur,r,f} + M_{m,r,f} + M_{p,r,f} = 392.2 \text{ kNm/m}$

Factored bearing pressure

Total moment for bearing; $M_{total,f} = M_{rest,f} - M_{ot,f} = 81.4 \text{ kNm/m}$
Total vertical reaction; $R_f = W_{total,f} = 212.7 \text{ kN/m}$
Distance to reaction; $x_{bar,f} = M_{total,f} / R_f = 383 \text{ mm}$
Eccentricity of reaction; $e_f = \text{abs}((l_{base} / 2) - x_{bar,f}) = 955 \text{ mm}$

Reaction acts outside middle third of base

Bearing pressure at toe; $p_{toe,f} = R_f / (1.5 \times x_{bar,f}) = 370.6 \text{ kN/m}^2$
Bearing pressure at heel; $p_{heel,f} = 0 \text{ kN/m}^2 = 0 \text{ kN/m}^2$
Rate of change of base reaction; $\text{rate} = p_{toe,f} / (3 \times x_{bar,f}) = 322.82 \text{ kN/m}^2/\text{m}$
Bearing pressure at stem / toe; $p_{stem_toe,f} = \text{max}(p_{toe,f} - (\text{rate} \times l_{toe}), 0 \text{ kN/m}^2) = 0 \text{ kN/m}^2$

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Bearing pressure at mid stem;

$$p_{\text{stem_mid_f}} = \max(p_{\text{toe_f}} - (\text{rate} \times (l_{\text{toe}} + t_{\text{wall}} / 2)), 0 \text{ kN/m}^2) = 0 \text{ kN/m}^2$$

Bearing pressure at stem / heel;

$$p_{\text{stem_heel_f}} = \max(p_{\text{toe_f}} - (\text{rate} \times (l_{\text{toe}} + t_{\text{wall}})), 0 \text{ kN/m}^2) = 0 \text{ kN/m}^2$$

Design of reinforced concrete retaining wall toe (BS 8002:1994)

Material properties

Characteristic strength of concrete;

$$f_{\text{cu}} = 40 \text{ N/mm}^2$$

Characteristic strength of reinforcement;

$$f_y = 500 \text{ N/mm}^2$$

Base details

Minimum area of reinforcement;

$$k = 0.13 \%$$

Cover to reinforcement in toe;

$$c_{\text{toe}} = 30 \text{ mm}$$

Calculate shear for toe design

Shear from bearing pressure;

$$V_{\text{toe_bear}} = 3 \times p_{\text{toe_f}} \times x_{\text{bar_f}} / 2 = 212.7 \text{ kN/m}$$

Shear from weight of base;

$$V_{\text{toe_wt_base}} = \gamma_{\text{f_d}} \times \gamma_{\text{base}} \times l_{\text{toe}} \times t_{\text{base}} = 15.3 \text{ kN/m}$$

Total shear for toe design;

$$V_{\text{toe}} = V_{\text{toe_bear}} - V_{\text{toe_wt_base}} = 197.4 \text{ kN/m}$$

Calculate moment for toe design

Moment from bearing pressure;

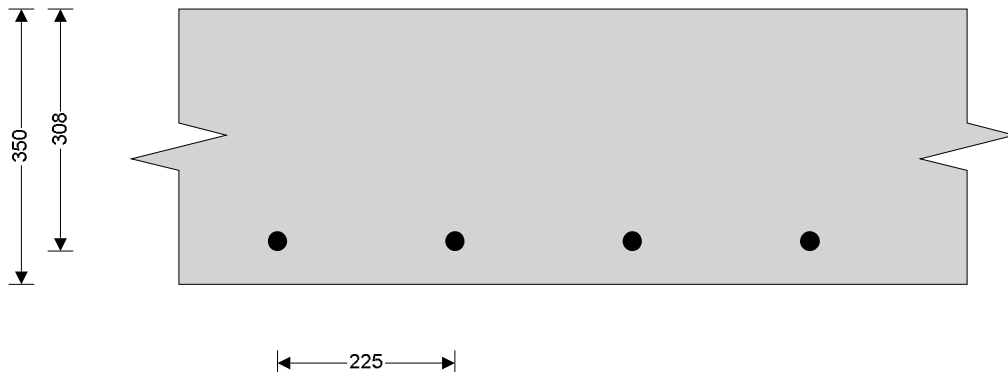
$$M_{\text{toe_bear}} = 3 \times p_{\text{toe_f}} \times x_{\text{bar_f}} \times (l_{\text{toe}} - x_{\text{bar_f}} + t_{\text{wall}} / 2) / 2 = 266.9 \text{ kNm/m}$$

Moment from weight of base;

$$M_{\text{toe_wt_base}} = (\gamma_{\text{f_d}} \times \gamma_{\text{base}} \times t_{\text{base}} \times (l_{\text{toe}} + t_{\text{wall}} / 2)^2 / 2) = 15.8 \text{ kNm/m}$$

Total moment for toe design;

$$M_{\text{toe}} = M_{\text{toe_bear}} - M_{\text{toe_wt_base}} = 251.1 \text{ kNm/m}$$



Check toe in bending

Width of toe;

$$b = 1000 \text{ mm/m}$$

Depth of reinforcement;

$$d_{\text{toe}} = t_{\text{base}} - c_{\text{toe}} - (\phi_{\text{toe}} / 2) = 307.5 \text{ mm}$$

Constant;

$$K_{\text{toe}} = M_{\text{toe}} / (b \times d_{\text{toe}}^2 \times f_{\text{cu}}) = 0.066$$

Compression reinforcement is not required

Lever arm;

$$z_{\text{toe}} = \min(0.5 + \sqrt{(0.25 - (\min(K_{\text{toe}}, 0.225) / 0.9))}, 0.95) \times d_{\text{toe}}$$

$$z_{\text{toe}} = 283 \text{ mm}$$

Area of tension reinforcement required;

$$A_{\text{s_toe_des}} = M_{\text{toe}} / (0.87 \times f_y \times z_{\text{toe}}) = 2041 \text{ mm}^2/\text{m}$$

Minimum area of tension reinforcement;

$$A_{\text{s_toe_min}} = k \times b \times t_{\text{base}} = 455 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required;

$$A_{\text{s_toe_req}} = \text{Max}(A_{\text{s_toe_des}}, A_{\text{s_toe_min}}) = 2041 \text{ mm}^2/\text{m}$$

Reinforcement provided;

25 mm dia.bars @ 225 mm centres

Area of reinforcement provided;

$$A_{\text{s_toe_prov}} = 2182 \text{ mm}^2/\text{m}$$

PASS - Reinforcement provided at the retaining wall toe is adequate

Check shear resistance at toe

Design shear stress;

$$v_{\text{toe}} = V_{\text{toe}} / (b \times d_{\text{toe}}) = 0.642 \text{ N/mm}^2$$

Allowable shear stress;

$$v_{\text{adm}} = \min(0.8 \times \sqrt{f_{\text{cu}} / 1 \text{ N/mm}^2}, 5) \times 1 \text{ N/mm}^2 = 5.000 \text{ N/mm}^2$$

PASS - Design shear stress is less than maximum shear stress

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From BS8110:Part 1:1997 – Table 3.8

Design concrete shear stress;

$$v_{c_toe} = 0.704 \text{ N/mm}^2$$

$v_{toe} < v_{c_toe}$ - No shear reinforcement required

Design of reinforced concrete retaining wall heel (BS 8002:1994)

Material properties

Characteristic strength of concrete;

$$f_{cu} = 40 \text{ N/mm}^2$$

Characteristic strength of reinforcement;

$$f_y = 500 \text{ N/mm}^2$$

Base details

Minimum area of reinforcement;

$$k = 0.13 \%$$

Cover to reinforcement in heel;

$$c_{heel} = 30 \text{ mm}$$

Calculate shear for heel design

Shear from weight of base;

$$V_{heel_wt_base} = \gamma_{fd} \times \gamma_{base} \times l_{heel} \times t_{base} = 8.2 \text{ kN/m}$$

Shear from weight of downstand;

$$V_{heel_wt_ds} = \gamma_{fd} \times \gamma_{base} \times d_{ds} \times t_{ds} = 6 \text{ kN/m}$$

Shear from weight of moist backfill;

$$V_{heel_wt_m} = w_{m_w_f} = 74.1 \text{ kN/m}$$

Shear from surcharge;

$$V_{heel_sur} = w_{sur_f} = 11.2 \text{ kN/m}$$

Total shear for heel design;

$$V_{heel} = V_{heel_wt_base} + V_{heel_wt_ds} + V_{heel_wt_m} + V_{heel_sur} = 99.6 \text{ kN/m}$$

Calculate moment for heel design

Moment from weight of base;

$$M_{heel_wt_base} = (\gamma_{fd} \times \gamma_{base} \times t_{base} \times (l_{heel} + t_{wall} / 2)^2 / 2) = 6.3 \text{ kNm/m}$$

Moment from weight of downstand;

$$M_{heel_wt_ds} = \gamma_{fd} \times \gamma_{base} \times d_{ds} \times t_{ds} \times (l_{ds} - l_{toe} + (t_{ds} - t_{wall}) / 2) = 1.1 \text{ kNm/m}$$

Moment from weight of moist backfill;

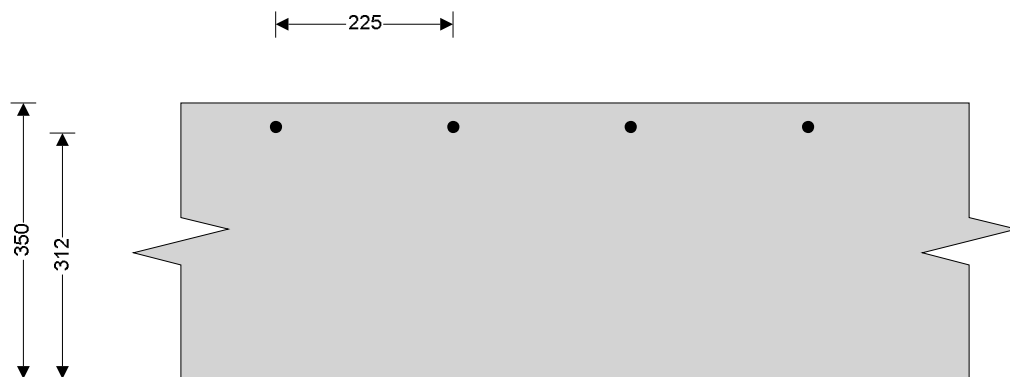
$$M_{heel_wt_m} = w_{m_w_f} \times (l_{heel} + t_{wall}) / 2 = 50.9 \text{ kNm/m}$$

Moment from surcharge;

$$M_{heel_sur} = w_{sur_f} \times (l_{heel} + t_{wall}) / 2 = 7.7 \text{ kNm/m}$$

Total moment for heel design;

$$M_{heel} = M_{heel_wt_base} + M_{heel_wt_ds} + M_{heel_wt_m} + M_{heel_sur} = 66.1 \text{ kNm/m}$$



Check heel in bending

Width of heel;

$$b = 1000 \text{ mm/m}$$

Depth of reinforcement;

$$d_{heel} = t_{base} - c_{heel} - (\phi_{heel} / 2) = 312.0 \text{ mm}$$

Constant;

$$K_{heel} = M_{heel} / (b \times d_{heel}^2 \times f_{cu}) = 0.017$$

Compression reinforcement is not required

Lever arm;

$$z_{heel} = \min(0.5 + \sqrt{(0.25 - (\min(K_{heel}, 0.225) / 0.9))}, 0.95) \times d_{heel}$$

$$z_{heel} = 296 \text{ mm}$$

Area of tension reinforcement required;

$$A_{s_heel_des} = M_{heel} / (0.87 \times f_y \times z_{heel}) = 513 \text{ mm}^2/\text{m}$$

Minimum area of tension reinforcement;

$$A_{s_heel_min} = k \times b \times t_{base} = 455 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required;

$$A_{s_heel_req} = \text{Max}(A_{s_heel_des}, A_{s_heel_min}) = 513 \text{ mm}^2/\text{m}$$

Reinforcement provided;

$$16 \text{ mm dia. bars @ 225 mm centres}$$

Area of reinforcement provided;

$$A_{s_heel_prov} = 894 \text{ mm}^2/\text{m}$$

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PASS - Reinforcement provided at the retaining wall heel is adequate

Check shear resistance at heel

Design shear stress;

$$v_{heel} = V_{heel} / (b \times d_{heel}) = 0.319 \text{ N/mm}^2$$

Allowable shear stress;

$$v_{adm} = \min(0.8 \times \sqrt{f_{cu}} / 1 \text{ N/mm}^2, 5) \times 1 \text{ N/mm}^2 = 5.000 \text{ N/mm}^2$$

PASS - Design shear stress is less than maximum shear stress

From BS8110:Part 1:1997 – Table 3.8

Design concrete shear stress;

$$v_{c_heel} = 0.518 \text{ N/mm}^2$$

$v_{heel} < v_{c_heel}$ - No shear reinforcement required

Design of reinforced concrete retaining wall downstand (BS 8002:1994)

Material properties

Characteristic strength of concrete;

$$f_{cu} = 40 \text{ N/mm}^2$$

Characteristic strength of reinforcement;

$$f_y = 500 \text{ N/mm}^2$$

Base details

Minimum area of reinforcement;

$$k = 0.13 \%$$

Cover to reinforcement in downstand;

$$c_{ds} = 30 \text{ mm}$$

Calculate shear for downstand design

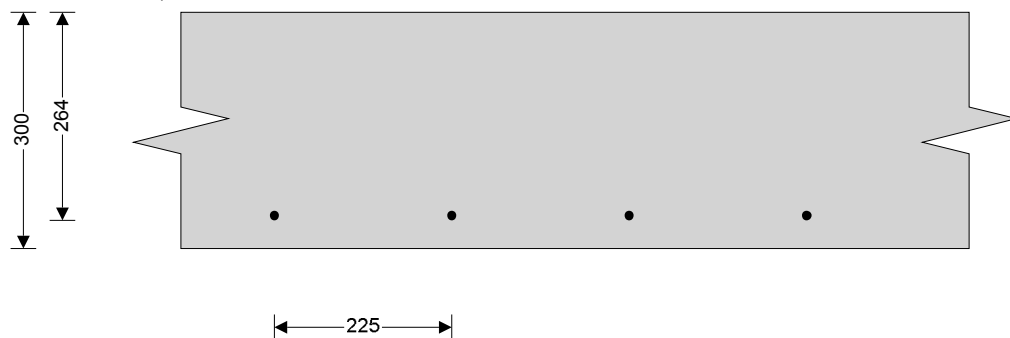
Total shear for downstand design

$$V_{down} = \gamma_{f_e} \times K_p \times \cos(\delta_b) \times \gamma_m \times d_{ds} \times (d_{cover} + t_{base} + d_{ds} / 2) = 62.4 \text{ kN/m}$$

Calculate moment for downstand design

Total moment for downstand design

$$M_{down} = \gamma_{f_e} \times K_p \times \cos(\delta_b) \times \gamma_m \times d_{ds} \times [(d_{cover} + t_{base}) \times (t_{base} + d_{ds}) + d_{ds} \times (t_{base} / 2 + 2 \times d_{ds} / 3)] / 2 = 31.9 \text{ kNm/m}$$



Check downstand in bending

Width of downstand;

$$b = 1000 \text{ mm/m}$$

Depth of reinforcement;

$$d_{down} = t_{ds} - c_{ds} - (\phi_{down} / 2) = 264.0 \text{ mm}$$

Constant;

$$K_{down} = M_{down} / (b \times d_{down}^2 \times f_{cu}) = 0.011$$

Compression reinforcement is not required

Lever arm;

$$z_{down} = \text{Min}(0.5 + \sqrt{(0.25 - (\min(K_{down}, 0.225) / 0.9))}, 0.95) \times d_{down}$$

$$z_{down} = 251 \text{ mm}$$

Area of tension reinforcement required;

$$A_{s_down_des} = M_{down} / (0.87 \times f_y \times z_{down}) = 292 \text{ mm}^2/\text{m}$$

Minimum area of tension reinforcement;

$$A_{s_down_min} = k \times b \times t_{ds} = 390 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required;

$$A_{s_down_req} = \text{Max}(A_{s_down_des}, A_{s_down_min}) = 390 \text{ mm}^2/\text{m}$$

Reinforcement provided;

$$12 \text{ mm dia. bars @ 225 mm centres}$$

Area of reinforcement provided;

$$A_{s_down_prov} = 503 \text{ mm}^2/\text{m}$$

PASS - Reinforcement provided at the retaining wall downstand is adequate

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Check shear resistance at downstand

Design shear stress;

$$v_{\text{down}} = V_{\text{down}} / (b \times d_{\text{down}}) = \mathbf{0.236 \text{ N/mm}^2}$$

Allowable shear stress;

$$v_{\text{adm}} = \min(0.8 \times \sqrt{f_{\text{cu}} / 1 \text{ N/mm}^2}, 5) \times 1 \text{ N/mm}^2 = \mathbf{5.000 \text{ N/mm}^2}$$

PASS - Design shear stress is less than maximum shear stress

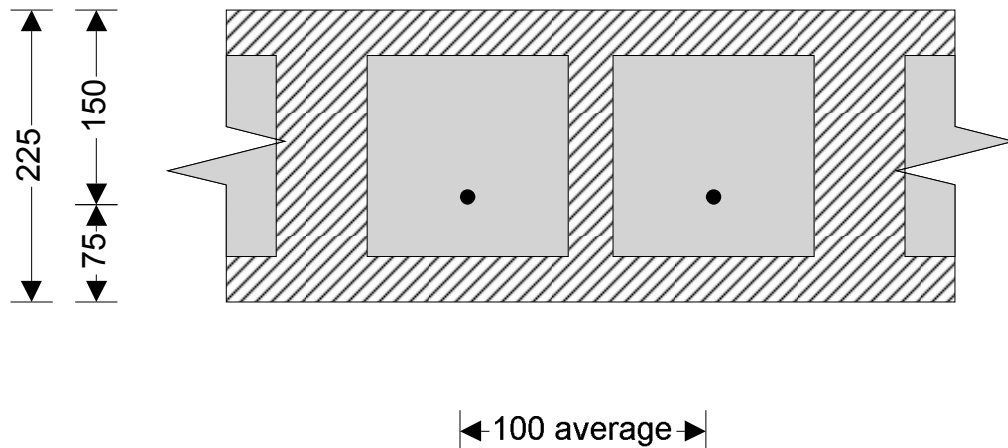
From BS8110:Part 1:1997 – Table 3.8

Design concrete shear stress;

$$v_{\text{c_down}} = \mathbf{0.472 \text{ N/mm}^2}$$

$v_{\text{down}} < v_{\text{c_down}}$ - No shear reinforcement required

Design of hollow reinforced masonry retaining wall stem - BS5628-2:2000



Wall details

Thickness of wall;

$$t_{\text{wall}} = \mathbf{225 \text{ mm}}$$

Depth of stem reinforcement;

$$d_{\text{stem}} = \mathbf{150 \text{ mm}}$$

Masonry details

Masonry type;

Aggregate concrete blocks no voids

Compressive strength of units;

$$p_{\text{unit}} = \mathbf{10.0 \text{ N/mm}^2}$$

Mortar designation;

(ii)

Category of manufacturing control of units;

Normal

Partial safety factor for material strength;

$$\gamma_{\text{mm}} = \mathbf{2.3}$$

Characteristic compressive strength of masonry

Least horizontal dimension of masonry units;

$$b_{\text{unit}} = \mathbf{100.0 \text{ mm}}$$

Height of masonry units;

$$h_{\text{unit}} = \mathbf{215.0 \text{ mm}}$$

Ratio of height to least horizontal dimension;

$$\text{ratio} = h_{\text{unit}} / b_{\text{unit}} = \mathbf{2.2}$$

From BS5628:2 Table 3d, mortar ii

Characteristic compressive strength;

$$f_k = \mathbf{8.1 \text{ N/mm}^2}$$

Factored horizontal at-rest forces on stem

Surcharge;

$$F_{\text{s_sur_f}} = \gamma_{\text{f_l}} \times K_0 \times \text{Surcharge} \times (h_{\text{eff}} - t_{\text{base}} - d_{\text{ds}}) = \mathbf{15.2 \text{ kN/m}}$$

Moist backfill above water table;

$$F_{\text{s_m_a_f}} = 0.5 \times \gamma_{\text{f_e}} \times K_0 \times \gamma_{\text{m}} \times (h_{\text{eff}} - t_{\text{base}} - d_{\text{ds}} - h_{\text{sat}})^2 = \mathbf{20.6 \text{ kN/m}}$$

Calculate shear for stem design

Shear at base of stem;

$$V_{\text{stem}} = F_{\text{s_sur_f}} + F_{\text{s_m_a_f}} = \mathbf{35.8 \text{ kN/m}}$$

Calculate moment for stem design

Surcharge;

$$M_{\text{s_sur}} = F_{\text{s_sur_f}} \times (h_{\text{stem}} + t_{\text{base}}) / 2 = \mathbf{15.8 \text{ kNm/m}}$$

Moist backfill above water table;

$$M_{\text{s_m_a}} = F_{\text{s_m_a_f}} \times (2 \times h_{\text{sat}} + h_{\text{eff}} - d_{\text{ds}} + t_{\text{base}}) / 3 = \mathbf{15.5 \text{ kNm/m}}$$

Total moment for stem design;

$$M_{\text{stem}} = M_{\text{s_sur}} + M_{\text{s_m_a}} = \mathbf{31.3 \text{ kNm/m}}$$

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Check maximum design moment for wall stem

Width of wall;

$$b = 1000 \text{ mm/m}$$

Maximum design moment;

$$M_{d_stem} = 0.4 \times f_k \times b \times d_{stem}^2 / \gamma_{mm} = 31.9 \text{ kNm/m}$$

PASS - Applied moment is less than maximum design moment

Check wall stem in bending

Moment of resistance factor;

$$Q = M_{stem} / d_{stem}^2 = 1.389 \text{ N/mm}^2$$

$$Q = 2 \times c \times (1 - c) \times f_k / \gamma_{mm}$$

$$c = 0.732$$

Lever arm factor;

$$Z_{stem} = \min(0.95, c) \times d_{stem} = 109.8 \text{ mm}$$

Lever arm;

$$A_{s_stem_des} = M_{stem} \times \gamma_{ms} / (f_y \times Z_{stem}) = 655 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required;

$$A_{s_stem_min} = k \times b \times t_{wall} = 293 \text{ mm}^2/\text{m}$$

Minimum area of tension reinforcement;

$$A_{s_stem_req} = \text{Max}(A_{s_stem_des}, A_{s_stem_min}) = 655 \text{ mm}^2/\text{m}$$

Area of tension reinforcement required;

16 mm dia.bars @ 225 mm centres

Reinforcement provided;

$$A_{s_stem_prov} = \pi \times \phi_{stem}^2 / (4 \times s_{stem}) = 905 \text{ mm}^2/\text{m}$$

Area of reinforcement provided;

PASS - Reinforcement provided at the retaining wall stem is adequate

Check shear resistance at wall stem

Design shear stress;

$$v_{stem} = V_{stem} / (b \times d_{stem}) = 0.239 \text{ N/mm}^2$$

Basic characteristic shear strength of masonry;

$$f_{vbas} = \min[0.35 + (17.5 \times A_{s_stem_prov} / (b \times d_{stem})), 0.7] \times 1 \text{ N/mm}^2$$

$$f_{vbas} = 0.482 \text{ N/mm}^2$$

Shear span;

$$a = M_{stem} / V_{stem} = 871.9 \text{ mm}$$

Characteristic shear strength of masonry;

$$f_v = \text{Min}(f_{vbas} \times \max(2.5 - 0.25 \times (a / d_{stem}), 1), 1.75 \text{ N/mm}^2)$$

$$f_v = 0.505 \text{ N/mm}^2$$

Allowable shear stress;

$$v_{adm} = f_v / \gamma_{mv} = 0.252 \text{ N/mm}^2$$

PASS - Design shear stress is less than maximum shear stress

Check limiting dimensions

Limiting span/effective depth ratio;

$$\text{ratio}_{max} = 18.00$$

Actual span/effective depth ratio;

$$\text{ratio}_{act} = (h_{stem} + d_{stem} / 2) / d_{stem} = 12.00$$

PASS - Span to depth ratio is acceptable

Axial load check

Factored axial load on wall;

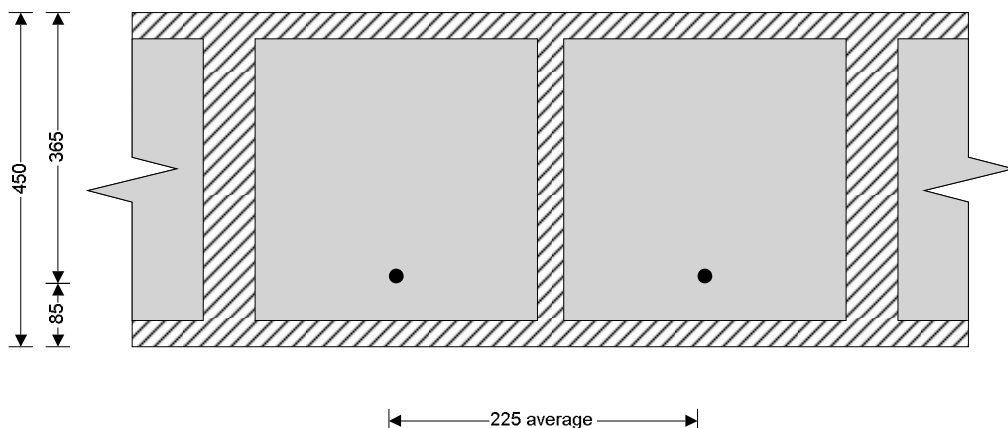
$$N_{wall} = ([t_{wall} \times h_{stem} \times \gamma_{wall} + W_{dead}] \times \gamma_{f_d}) + (W_{live} \times \gamma_{f_l}) = 11.4 \text{ kN/m}$$

Limiting axial load;

$$N_{limit} = 0.1 \times f_k \times t_{wall} = 183.2 \text{ kN/m}$$

Applied axial load may be ignored - calculations valid

DESIGN OF HOLLOW REINFORCED MASONRY RETAINING WALL STEM - BS5628-2:2000



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Wall details

Thickness of wall; $t_{wall} = 450 \text{ mm}$
Depth of stem reinforcement; $d_{stem} = 365 \text{ mm}$

Masonry details

Masonry type; **Aggregate concrete blocks no voids**
Compressive strength of units; $p_{unit} = 10.0 \text{ N/mm}^2$
Mortar designation; **(ii)**
Category of manufacturing control of units; **Normal**
Partial safety factor for material strength; $\gamma_{mm} = 2.3$

Characteristic compressive strength of masonry

Least horizontal dimension of masonry units; $b_{unit} = 100.0 \text{ mm}$
Height of masonry units; $h_{unit} = 215.0 \text{ mm}$
Ratio of height to least horizontal dimension; $ratio = h_{unit} / b_{unit} = 2.2$

From BS5628:2 Table 3d, mortar ii

Characteristic compressive strength; $f_k = 8.1 \text{ N/mm}^2$

Factored horizontal at-rest forces on stem

Surcharge; $F_{s_sur_f} = \gamma_{t_j} \times K_0 \times \text{Surcharge} \times (h_{eff} - t_{base} - d_{ds}) = 27.1 \text{ kN/m}$
Moist backfill above water table; $F_{s_m_a_f} = 0.5 \times \gamma_{t_e} \times K_0 \times \gamma_m \times (h_{eff} - t_{base} - d_{ds} - h_{sat})^2 = 65.6 \text{ kN/m}$

Calculate shear for stem design

Shear at base of stem; $V_{stem} = F_{s_sur_f} + F_{s_m_a_f} = 92.7 \text{ kN/m}$

Calculate moment for stem design

Surcharge; $M_{s_sur} = F_{s_sur_f} \times (h_{stem} + t_{base}) / 2 = 46.4 \text{ kNm/m}$
Moist backfill above water table; $M_{s_m_a} = F_{s_m_a_f} \times (2 \times h_{sat} + h_{eff} - d_{ds} + t_{base} / 2) / 3 = 78.7 \text{ kNm/m}$
Total moment for stem design; $M_{stem} = M_{s_sur} + M_{s_m_a} = 125.1 \text{ kNm/m}$

Check maximum design moment for wall stem

Width of wall; $b = 1000 \text{ mm/m}$
Maximum design moment; $M_{d_stem} = 0.4 \times f_k \times b \times d_{stem}^2 / \gamma_{mm} = 188.6 \text{ kNm/m}$
PASS - Applied moment is less than maximum design moment

Check wall stem in bending

Moment of resistance factor; $Q = M_{stem} / d_{stem}^2 = 0.939 \text{ N/mm}^2$
 $Q = 2 \times c \times (1 - c) \times f_k / \gamma_{mm}$
Lever arm factor; $c = 0.843$
Lever arm; $z_{stem} = \min(0.95, c) \times d_{stem} = 307.5 \text{ mm}$
Area of tension reinforcement required; $A_{s_stem_des} = M_{stem} \times \gamma_{ms} / (f_y \times z_{stem}) = 936 \text{ mm}^2/\text{m}$
Minimum area of tension reinforcement; $A_{s_stem_min} = k \times b \times t_{wall} = 585 \text{ mm}^2/\text{m}$
Area of tension reinforcement required; $A_{s_stem_req} = \text{Max}(A_{s_stem_des}, A_{s_stem_min}) = 936 \text{ mm}^2/\text{m}$
Reinforcement provided; **20 mm dia.bars @ 225 mm centres**
Area of reinforcement provided; $A_{s_stem_prov} = \pi \times \phi_{stem}^2 / (4 \times s_{stem}) = 1396 \text{ mm}^2/\text{m}$

PASS - Reinforcement provided at the retaining wall stem is adequate

Check shear resistance at wall stem

Design shear stress; $v_{stem} = V_{stem} / (b \times d_{stem}) = 0.254 \text{ N/mm}^2$
Basic characteristic shear strength of masonry; $f_{vbas} = \min[0.35 + (17.5 \times A_{s_stem_prov} / (b \times d_{stem})), 0.7] \times 1 \text{ N/mm}^2$
 $f_{vbas} = 0.417 \text{ N/mm}^2$
Shear span; $a = M_{stem} / V_{stem} = 1349.8 \text{ mm}$

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Characteristic shear strength of masonry;

$$f_v = \text{Min}(f_{vbas} \times \max(2.5 - 0.25 \times (a / d_{stem}), 1), 1.75 \text{ N/mm}^2)$$

$$f_v = \mathbf{0.657 \text{ N/mm}^2}$$

Allowable shear stress;

$$v_{adm} = f_v / \gamma_{mv} = \mathbf{0.328 \text{ N/mm}^2}$$

PASS - Design shear stress is less than maximum shear stress

Check limiting dimensions

Limiting span/effective depth ratio;

$$\text{ratio}_{\max} = \mathbf{18.00}$$

Actual span/effective depth ratio;

$$\text{ratio}_{\text{act}} = (h_{stem} + d_{stem} / 2) / d_{stem} = \mathbf{8.92}$$

PASS - Span to depth ratio is acceptable

Axial load check

Factored axial load on wall;

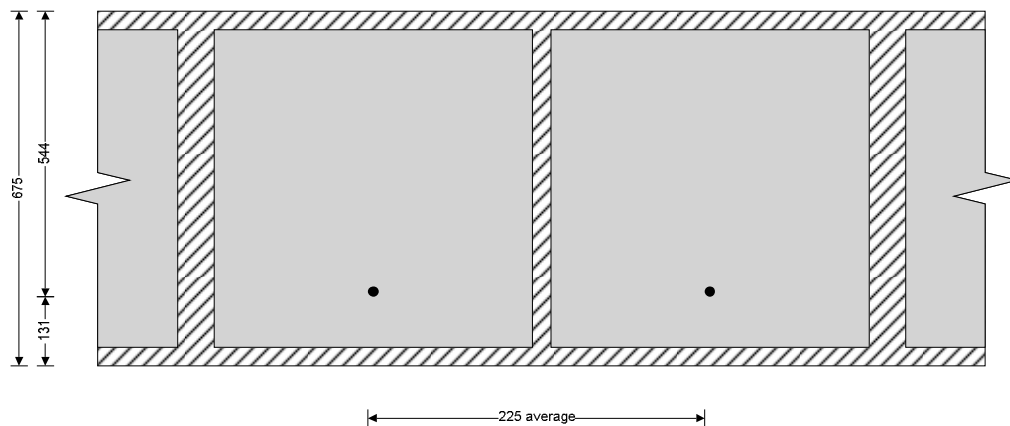
$$N_{wall} = ([t_{wall} \times h_{stem} \times \gamma_{wall} + W_{dead}] \times \gamma_{f,d}) + (W_{live} \times \gamma_{f,l}) = \mathbf{40.7 \text{ kN/m}}$$

Limiting axial load;

$$N_{limit} = 0.1 \times f_k \times t_{wall} = \mathbf{366.4 \text{ kN/m}}$$

Applied axial load may be ignored - calculations valid

Design of hollow reinforced masonry retaining wall stem - BS5628-2:2000



Wall details

Thickness of wall;

$$t_{wall} = \mathbf{675 \text{ mm}}$$

Depth of stem reinforcement;

$$d_{stem} = \mathbf{544 \text{ mm}}$$

Masonry details

Masonry type;

Aggregate concrete blocks no voids

Compressive strength of units;

$$p_{unit} = \mathbf{10.0 \text{ N/mm}^2}$$

Mortar designation;

(ii)

Category of manufacturing control of units;

Normal

Partial safety factor for material strength;

$$\gamma_{mm} = \mathbf{2.3}$$

Characteristic compressive strength of masonry

Least horizontal dimension of masonry units;

$$b_{unit} = \mathbf{100.0 \text{ mm}}$$

Height of masonry units;

$$h_{unit} = \mathbf{215.0 \text{ mm}}$$

Ratio of height to least horizontal dimension;

$$\text{ratio} = h_{unit} / b_{unit} = \mathbf{2.2}$$

From BS5628:2 Table 3d, mortar ii

Characteristic compressive strength;

$$f_k = \mathbf{8.1 \text{ N/mm}^2}$$

Factored horizontal at-rest forces on stem

Surcharge;

$$F_{s_sur_f} = \gamma_{f,l} \times K_0 \times \text{Surcharge} \times (h_{eff} - t_{base} - d_{ds}) = \mathbf{37 \text{ kN/m}}$$

Moist backfill above water table;

$$F_{s_m_a_f} = 0.5 \times \gamma_{f,e} \times K_0 \times \gamma_m \times (h_{eff} - t_{base} - d_{ds} - h_{sat})^2 = \mathbf{122.4 \text{ kN/m}}$$

Calculate shear for stem design

Shear at base of stem;

$$V_{stem} = F_{s_sur_f} + F_{s_m_a_f} = \mathbf{159.4 \text{ kN/m}}$$

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Calculate moment for stem design

Surcharge;

$$M_{s_sur} = F_{s_sur_f} \times (h_{stem} + t_{base}) / 2 = \mathbf{84.2 \text{ kNm/m}}$$

Moist backfill above water table;

$$M_{s_m_a} = F_{s_m_a_f} \times (2 \times h_{sat} + h_{eff} - d_{ds} + t_{base} / 2) / 3 = \mathbf{192.8 \text{ kNm/m}}$$

Total moment for stem design;

$$M_{stem} = M_{s_sur} + M_{s_m_a} = \mathbf{277 \text{ kNm/m}}$$

Check maximum design moment for wall stem

Width of wall;

$$b = \mathbf{1000 \text{ mm/m}}$$

Maximum design moment;

$$M_{d_stem} = 0.4 \times f_k \times b \times d_{stem}^2 / \gamma_{mm} = \mathbf{419.0 \text{ kNm/m}}$$

PASS - Applied moment is less than maximum design moment

Check wall stem in bending

Moment of resistance factor;

$$Q = M_{stem} / d_{stem}^2 = \mathbf{0.936 \text{ N/mm}^2}$$

$$Q = 2 \times c \times (1 - c) \times f_k / \gamma_{mm}$$

Lever arm factor;

$$c = \mathbf{0.843}$$

Lever arm;

$$z_{stem} = \min(0.95, c) \times d_{stem} = \mathbf{458.7 \text{ mm}}$$

Area of tension reinforcement required;

$$A_{s_stem_des} = M_{stem} \times \gamma_{ms} / (f_y \times z_{stem}) = \mathbf{1389 \text{ mm}^2/\text{m}}$$

Minimum area of tension reinforcement;

$$A_{s_stem_min} = k \times b \times t_{wall} = \mathbf{878 \text{ mm}^2/\text{m}}$$

Area of tension reinforcement required;

$$A_{s_stem_req} = \text{Max}(A_{s_stem_des}, A_{s_stem_min}) = \mathbf{1389 \text{ mm}^2/\text{m}}$$

Reinforcement provided;

$$\mathbf{20 \text{ mm dia. bars @ 225 mm centres}}$$

Area of reinforcement provided;

$$A_{s_stem_prov} = \pi \times \phi_{stem}^2 / (4 \times s_{stem}) = \mathbf{1396 \text{ mm}^2/\text{m}}$$

PASS - Reinforcement provided at the retaining wall stem is adequate

Check shear resistance at wall stem

Design shear stress;

$$v_{stem} = V_{stem} / (b \times d_{stem}) = \mathbf{0.293 \text{ N/mm}^2}$$

Basic characteristic shear strength of masonry;

$$f_{vbas} = \min[0.35 + (17.5 \times A_{s_stem_prov} / (b \times d_{stem})), 0.7] \times 1 \text{ N/mm}^2$$

$$f_{vbas} = \mathbf{0.395 \text{ N/mm}^2}$$

Shear span;

$$a = M_{stem} / V_{stem} = \mathbf{1737.5 \text{ mm}}$$

Characteristic shear strength of masonry;

$$f_v = \text{Min}(f_{vbas} \times \max(2.5 - 0.25 \times (a / d_{stem}), 1), 1.75 \text{ N/mm}^2)$$

$$f_v = \mathbf{0.672 \text{ N/mm}^2}$$

Allowable shear stress;

$$v_{adm} = f_v / \gamma_{mv} = \mathbf{0.336 \text{ N/mm}^2}$$

PASS - Design shear stress is less than maximum shear stress

Check limiting dimensions

Limiting span/effective depth ratio;

$$\text{ratio}_{max} = \mathbf{18.00}$$

Actual span/effective depth ratio;

$$\text{ratio}_{act} = (h_{stem} + d_{stem} / 2) / d_{stem} = \mathbf{8.22}$$

PASS - Span to depth ratio is acceptable

Axial load check

Factored axial load on wall;

$$N_{wall} = ([t_{wall} \times h_{stem} \times \gamma_{wall} + W_{dead}] \times \gamma_{f_d}) + (W_{live} \times \gamma_{f_l}) = \mathbf{83.3 \text{ kN/m}}$$

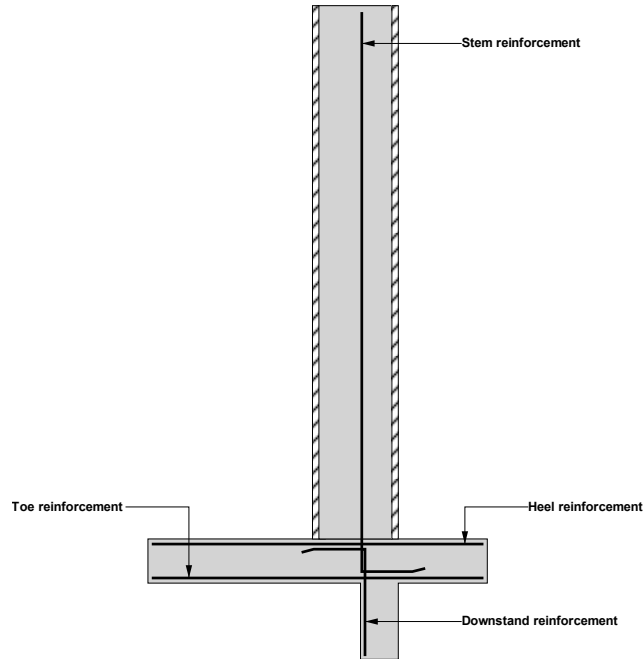
Limiting axial load;

$$N_{limit} = 0.1 \times f_k \times t_{wall} = \mathbf{549.6 \text{ kN/m}}$$

Applied axial load may be ignored - calculations valid

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Indicative retaining wall reinforcement diagram



Toe bars - 25 mm dia.@ 225 mm centres - (2182 mm²/m)
 Heel bars - 16 mm dia.@ 225 mm centres - (894 mm²/m)
 Downstand bars - 12 mm dia.@ 225 mm centres - (503 mm²/m)
 Stem bars - 20 mm dia.@ 225 mm centres - (1396 mm²/m)